# Traffic and Highways Report 

Land off Foxlydiate Lane, Webheath, Redditch
Heyford Developments Ltd

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## 1 Introduction

Halcrow Group Ltd has been commissioned by Heyford Developments Ltd to produce a report to present the opportunities associated with development of land off Foxlydiate Lane in Redditch.

This report considers how local public transport, the sustainable transport network and highway network is able to support this development.

### 1.1 Background

The proposed development site is located approximately three kilometres to the west of Redditch town centre, on the administrative boundary between Redditch Borough and Bromsgrove District.

The site is bounded by the A448 Bromsgrove Highway to the north, Foxlydiate Lane to the east and Curr Lane to the south. The site extends to provide 41 hectares of developable land. The site could provide up to 1,400 residential units and associated land uses.


Figure 1.1 - Site Location

## 2 Sustainable Transport Network

### 2.1 Introduction

The development site is located to the south of the A448 Bromsgrove Highway, on the west side of the Redditch urban area. The A448 provides access into Redditch town centre and North Redditch to the east and Bromsgrove, the M42 and the M5 to the west. The site is fully accessible to local amenities and the town centre, which is within an acceptable travel distance by sustainable and 'private car' modes.

This section of the report outlines the sustainable transport network that serves the site.

### 2.2 Pedestrian Access

The site has a good level of access to the local footpath network, with bridges and subways providing links across the A448 Bromsgrove Highway. The local roads are relatively lightly trafficked, which makes for pleasant and safe passage to local amenities.

The local nursery is 1.2 km from the site, adjacent to the local shop. The local dentist is only a 5 minute walk from the site.

Two local primary schools on the Webheath estate are only a 12 minute walk ( 850 m ) from the edge of the site, and crossing wardens are present to aid parents and children. Schools to the north of the A448 are only a 1.3 km walk from the site, via subways or bridges.

### 2.3 Cycle Access

The WCC Local Transport Plan 2 indicates that the standard length of cycle journey is 5 km . Furthermore PPG 13 states that cycling has the potential to substitute short car trips, particularly those less than 5 km and forming part of a longer journey by public transport.

The site is less than 500 m from National Cycle Network Route 5, which connects the site to the local primary schools, the doctor's surgery, the Pitcheroak Secondary School, North East Worcestershire College, and Trinity High School.


Figure 2.1: Existing cycle access
Church Road has been identified as a Redditch area cycle route; this provides a direct connection from the development site to the National Cycle Network. A bridleway also crosses the site from the A448 junction with Birchfield Road to Curr Lane.


The Colleges and sixth form centres are located in, or on the way to, Redditch town centre, as is the Railway Station. This is only a 10 minute $(2.4 \mathrm{~km})$ cycle ride, which means that all retail, education, primary health amenities and most employment in Redditch is within standard cycle distance of the site.

Census (2001) Journey to Work data suggests that around $2 \%$ of people who live to the west of the town cycle to work. Only 5 km from the site is a large employment area in Redditch including Alexandra Hospital. It is therefore considered that both healthcare and employment is accessible from the site by bicycle.

It can be concluded that the site is within a reasonable cycle journey of healthcare, retail, education and employment uses within Redditch, and, in accordance with PPG13, is within a viable multi-model journey of Redditch Railway Station.

### 2.4 Public Transport

The site is directly served by 13 buses per hour providing a 20 minute bus journey to Redditch. Buses connect the site with Redditch Railway Station (a 2.5 km walk), which provides two services an hour into Birmingham New Street, on the Cross City line.

The site is located directly adjacent to Foxlydiate Lane and Birchfield Road, which are served by the 68, 143 and 343, and Foxlydiate Crescent, served by the 50 and 51.
Within 800 m walk of the site there are at least 13 buses an hour providing a 10 minute journey time into the centre of Redditch, and the Railway Station.


Figure 2.2: Existing bus access
The 50 and 51 run from Foxlydiate Crescent, seven minutes ( 600 m ) walk away, to provide ten buses per hour into Redditch, with an on-bus journey time of ten minutes. The 68 provides three buses per hour from Tynsall Avenue (500m) providing less than a ten minute journey time into Redditch.

The S 69 service provides a 20 minute journey on a school bus from the site to Ridgeway Middle School. There are also less frequent services that connect the site to Bromsgrove and Birmingham - 143 and 334 hourly ( 250 m away).

A number of services connect the bus station to Alexandra Hospital, with a ten minute journey time. It is anticipated that the journey time to the hospital from Foxlydiate could be as little at 20 minutes and at the most, 60 minutes.

The site is within a short bus journey of schools, colleges, GP surgeries, shops, the Railway Station, jobs and the hospital. Census reports that $7 \%$ of local people choose to catch the bus to work.


Figure 2.3: Existing bus services within Redditch
There are half hourly train services from Redditch Railway Station into Longbridge, University (Queen Elizabeth Hospital), Birmingham New Street and beyond. The station is located close to the town centre and bus station and provides over 150 car parking spaces.

The Railway Station is a ten minute cycle or bus ride away from the site $(2.5 \mathrm{~km})$, which is within the acceptable catchment area of the station. This is further reinforced by PPG 13, which states that cycling has the potential to substitute short car trips, particularly those under 5 km and form part of a longer journey by public transport. Census 2001 reports only $1 \%$ of local people use the railway to travel to work.

### 2.5 Opportunities

The proposed site will be designed to be permeable, and safe crossings of local roads will be provided where appropriate to remove severance to local facilities. The walk routes to local schools, which are already convenient and mostly along lightly trafficked routes, can be improved to deliver safer journeys for children and parents.

The development site could provide a local centre, accommodating shops, nurseries and healthcare, which would reduce the need to travel outside of the site, and encourage more sustainable short journeys.

Signed cycle routes can be provided within the site and the connection to National Cycle Network Route 5 could be improved, providing safer routes to schools, college, Redditch town centre and the Railway Station.

A new bus service (or an enhancement of an existing service) could be provided, to penetrate the Foxlydiate site, serving the Webheath estate, and the Webheath ADR land, connecting to Redditch town centre and all the schools on the north side of the A448. Gold and Silver standard bus shelters will be provided in the locality to
improve the waiting environment for passengers. This could potentially bring enhanced accessibility for future and existing residents of the area.


Figure 2.4: Proposed new service (linking new developments in Webheath)
Access to the Railway Station will also be improved by the new bus service and connection to NCN Route 5.

### 2.6 Summary

The site is ideally located for walk journeys to schools and shops. Walk routes are generally pleasant, safe and lightly trafficked. The site will be designed to be permeable and will provide amenities on site to encourage shorter and more sustainable journeys, whilst also reducing the need to travel.

The site is ideally located for journeys to local schools, jobs, healthcare, colleges, Redditch town centre and the Railway Station. The existing facilities will be significantly enhanced by providing connections into the important National Cycle Network, which passes the site and by having a site layout that is sustainable by design.

The site provides access to healthcare, employment, retail, educational and leisure facilities within 30-60 minutes using a conventional bus. The site is within reach, but not ideally located in relation to the local high frequency bus services. Therefore a bus can be provided to complement these services and permeate the site to ensure that occupiers will be within 250 m of a stop. Stops will be provided to a high standard.

The site benefits from its good connections to the Railway Station, which is only ten minutes away by cycle or by bus.

Opportunities to improve the sustainable access of the site include: a new (or enhanced) bus service linking new developments within Webheath to Redditch Town Centre and Railway Station; signed cycle routes to NCN 5 and potentially providing a local centre within the development to improve access to amenities by foot and cycle, as well as reducing the need to travel.

## 3 Local Highway Network

### 3.1 Introduction

Redditch is well served by a network of high quality strategic roads including the A435, A441 and A448. These roads are free flowing and are rarely subject to significant delays or congestion. This means that roads off the A448, such as Birchfield Road adjacent to the site, do not suffer the effects of through traffic like other towns in the county. The observed traffic conditions confirm the LTP2 (Worcestershire Local Transport Plan 2, 2006-2011) assertion that there are relatively few problems relating to traffic congestion.

The A448 Bromsgrove Highway provides the site with excellent connections to Redditch, Bromsgrove, Stratford, Evesham and the Strategic Road Network. There are no congestion problems on this de-restricted dual carriageway road, and it is observed that the junction to this development has the capacity to cater for development in the area. The development is therefore ideally located on the periphery of Redditch in order to benefit from the existing highway network.

Foxlydiate Road, Church Road and Heathfield Road provide distributor network for this development at Foxlydiate. These routes are between 5.5 m and 6.5 m , with driveway access, and parking along their length.

Access to the town centre and beyond is good with free flowing routes via the A441 Alvechurch Highway, Bromsgrove Road, and the B4184 Brockhill Drive.

### 3.2 Existing Highway Network

The current highway network adjacent to the site is shown in Halcrow drawing no: CTB-AOE-0001 (Appendix C). The site is bounded to the north-west by the elevated A448/Birchfield Road overbridge and to the east by Foxlydiate Lane. The configuration of the over-bridge and the proximity of the A448/Birchfield Road priority junction are constraints to providing access to the site in this location without modification of the layout.

### 3.3 Traffic Surveys

Traffic surveys were undertaken at three junctions adjacent to the site (September 2010) in order to establish current traffic flows on the network. This data forms the background for future development traffic flow calculations and modelling. Figure 3.1 overleaf illustrates the location of the surveyed junctions.

Table 3.1 summarises the traffic flows for the morning peak period (08:00-09:00; these flows have been reported as they are higher than the PM peak flows. Full disaggregated traffic flows (for AM and PM periods) are available in Appendix A.


Figure 3.1: Junction locations for traffic surveys
It has been observed that there are currently no congestion issues regarding the three junctions surrounding the development site, and all junctions operate within capacity.

| Road | Survey Flow |
| :--- | ---: |
| Junction One |  |
| Birchfield Road (to A448 W/bound) | 167 |
| Birchfield Road | 364 |
| Birchfield Road (to A448 E/bound) | 581 |
| Total | $\mathbf{1 1 1 2}$ |
| Junction Two |  |
| Birchfield Road (East) | 272 |
| Foxlydiate Lane | 147 |
| Birchfield Road (West) | 284 |
| Total | 703 |
| Junction Three |  |
| Foxlydiate Lane | 86 |
| Church Road | 101 |
| Great Hockings Lane | 81 |
| Curr Lane | 32 |
| Total | $\mathbf{3 0 0}$ |


| Road | Survey Flow |
| :--- | ---: |
| Junction Four (new junction - for comparison) |  |
| Birchfield Road (to A448 E/bound) | 581 |
| Birchfield Road (to A448 W/bound) | 383 |
| Development Link Road | 0 |
| Development Access | 0 |
| Total | $\mathbf{9 6 4}$ |

Table 3.1: Background traffic flows (AM peak - 08:00 - 09:00)

### 3.4 Accident Analysis

An accident analysis has been undertaken for the key links and junctions adjacent to the site using data obtained from Worcestershire County Council for the past five years (2005-2010). The data shows that twelve reported accidents have taken place during the review period. Full details of the accident data can be found in Appendix B.

Ten of these accidents were slight, one serious and one was fatal. Figure 3.2 illustrates the location and severity of these accidents. When investigating the causation factors of these accidents, no accidents were caused by the same factor.


Figure 3.2: Accidents within the vicinity of the development site (past five years)
Table 3.2 provides further details of the accidents that occurred within the survey period, 2005-2010. It shows that although several of the slight accidents on the B4096 junction were shunt incidents, no two factors resulting in the accidents were the same. The serious incident was caused by low sun obstructing the driver's view of the road and the fatal accident was between a vehicle and a pedestrian crossing the A448.

Table 3.2: Accident survey data (2005-2010)
$\left.\begin{array}{|l|l|l|}\hline \text { Type } & \text { Location } & \text { Causation Factor } \\ \hline \text { Fatal } & \begin{array}{l}\text { A448 (eastbound) near } \\ \text { junction with B4096 }\end{array} & \text { Vehicle collided with pedestrian } \\ \hline \text { Serious } & \begin{array}{l}\text { Birchfield Road, junction } \\ \text { with Reynard Close }\end{array} & \begin{array}{l}\text { Vehicles 1 \& 2 travelling toward } \\ \text { each on opposite sides of } \\ \text { carriageway, for unknown } \\ \text { reason Vehicle 1 travelled into } \\ \text { opposite side of carriageway and } \\ \text { collided with Vehicle 2. }\end{array} \\ \hline \text { Slight } & \begin{array}{l}\text { Birchfield Road, junction } \\ \text { with Foxlydiate Lane }\end{array} & \begin{array}{l}\text { Low sun blinded driver, collision } \\ \text { with vehicle in opposite } \\ \text { carriageway. }\end{array} \\ \hline \text { Slight } & \begin{array}{l}\text { A448 (eastbound), approx } \\ 160 \mathrm{~m} \text { south-east of junction } \\ \text { with Birchfield Road. }\end{array} & \begin{array}{l}\text { Icy road surface, vehicle collision } \\ \text { (2 vehicles involved) }\end{array} \\ \hline \text { Slight } & \begin{array}{l}\text { A448 (eastbound), approx } \\ 90 m \text { south-east of junction } \\ \text { with Birchfield Road. }\end{array} & \begin{array}{l}\text { Icy road surface - vehicle leaves } \\ \text { carriageway (nearside) into } \\ \text { trees. }\end{array} \\ \hline \text { Slight } & \begin{array}{l}\text { B4096 Hewell Lane RBT, } \\ \text { junction with B4184 } \\ \text { Bromsgrove }\end{array} & \begin{array}{l}\text { Vehicle 1 stops for traffic, struck } \\ \text { in rear by Vehicle 2. }\end{array} \\ \hline \text { Slight } & \begin{array}{l}\text { Slight }\end{array} & \begin{array}{l}\text { B4096 Hewell Lane, } \\ \text { junction with B4184 }\end{array} \\ \hline \text { B4096 Hewell Lane RBT, } \\ \text { junction with B4096 } \\ \text { Birchfield Road }\end{array} \quad \begin{array}{l}\text { 9-year passenger not fitted with } \\ \text { seatbelt; driver not paying } \\ \text { attention - brakes suddenly and } \\ \text { passenger hurts head. }\end{array}\right\}$

### 3.5 Mode split and existing traffic distribution

Journey to Work (Census 2001) data has been analysed to understand the current mode split of residents from the residential areas adjacent to the site. This information will be used to inform the assumptions made for the mode split and distribution of development traffic. Table 3.3 outlines the mode split of the residential areas surrounding the development site.

Table 3.3: Current mode split

| Mode | Percentage |
| :--- | ---: |
| Rail |  |
| Bus | 1 |
| Walking | 6.5 |
| Cycling | 8 |
| Car (Drive) | 1.5 |
| Car (Passenger) | 75 |
| Motorcycle | 7 |

The census data has also been plotted in GIS to understand the origin/destinations of work trips. The data indicates that $52 \%$ of residents work within Redditch; 19\% of which work in Redditch (south of the A448) in areas around the Hospital, Park Farm and Washford; the other $33 \%$ of people work in Redditch north of the A448, which includes Redditch town centre. $48 \%$ of people work outside of the Redditch area.

### 3.6 Proposed Access Strategy

The proposed access strategy for the site will provide two access points; the first formed at the A448/Birchfield Road over-bridge via a new four-arm roundabout; the second via a new link to Curr Lane. The proposed access arrangements are shown in Halcrow drawing no's: CTB-AOE-0002, CTB-AOE-0003 and CTB-AOE-0004
(Appendix C). It should be noted that the alignment of the link road is indicative only at this stage; this will be subject to refinement during detailed design.

The access arrangements have been designed in accordance with the relevant design guidance and incorporate the following features:

- Re-alignment of A448/Birchfield Road carriageway to the south of the A448 to link to a new four-arm roundabout approximately 40 metres in diameter.
- Roundabout has been designed in accordance with DMRB TD16/07 "Geometric Design of Roundabouts".
- Residential Distributor Road connecting new A448 roundabout with Curr Lane. Link design provided in accordance with Worcestershire County Council's "Highway Design Guide for New Developments".
- Re-alignment of Curr Lane to connect with Residential Distributor Road at new priority junction. New junction designed in accordance with TD 92/95 "Geometric Design of Major/Minor junctions".
- Relevant earthworks have been identified and these are based upon a 1:2 slope.

It is anticipated that the provision of a new distributor road from the A448 to Curr Lane may lead to a re-assignment of background and other future development traffic, from lower grade local roads such as Foxlydiate Road, Church Road and Heathfield Road.

### 3.7 Stage One Road Safety Audit

Road Safety Audits provide an evaluation of highway improvement schemes during design and at the end of construction to identify potential road safety problems that may affect any users of the highway and to suggest measures to eliminate or mitigate these problems.

The preliminary design for the proposed access arrangements is shown in Halcrow drawing no: CTBAOE-OO2 (Appendix C). In accordance with the guidance set out in the Design Manual for Roads and Bridges (DMRB), Volume 5, Section 2, HD19/03, a Stage One Road Safety Audit has been undertaken. The primary purpose of the audit is to identify any road safety issues which may require mitigation. This is important as it allows all land requirements to be identified at this early stage, thus it can be demonstrated that a safe junction can be provided without a requirement for third party land.

A full copy of the Road Safety Audit is provided in Appendix D, a summary of the issues raised is provided below:

- Space for safety fencing on northern roundabout: As the proposed road is to serve a residential development, the roundabout is likely to have a footway behind the kerb lines. The roundabout is situated on a high embankment and thus vehicular safety fencing is likely to be required behind the footway.
- Severe entry deflections on northern roundabout: The SW, NW and NE entry arms have severe entry deflections without any transitional curve between the entry straight and entry radius. The driven entry reflection is much lower than 100 m (the allowable maximum). The alignments approaching the roundabout are fairly straight and could permit high speeds. These arrangments could lead to loss of control type accidents.
- Potential for shunt accidents at new Curr Lane junction: The " T " junction is located mid-way around the 80 m radius curve and could lead to southbound shunt accidents into vehicles waiting to turn right onto Curr Lane.

The Designer's response to the issues raised is summarised below:

- Space for safety fencing on northern roundabout: Problem accepted - provision for footway and safety fence to be incorporated in detailed design.
- Severe entry deflections on northern roundabout: Problem accepted - alignments to be repositioned in detailed design.
- Potential for shunt accidents at new Curr Lane junction: Problem accepted AADT flows to be interrogated to decide upon form of junction.

It is clear from this process that all issues raised can be incorporated within the detailed design, thus at this stage there are not considered to be any highway safety reasons which would preclude the implementation of this junction.

### 3.8 Summary

Currently there are no capacity issues regarding the three junctions surrounding the development site, with all junctions working within their capacity. Access to

Redditch town centre and beyond is good with free flowing routes via the A441 Alvechurch Highway, Bromsgrove Road, and the B4184 Brockhill Drive.

This section has also presented the proposed access vehicular access strategy for the site which comprises two new access points and the provision of a link road between the A448 and Curr Lane.

The scheme has been designed in accordance with the relevant design criteria and a Stage One Road safety Audit has also been undertaken. It has been demonstrated that access to the site can be provided which is safe, in accordance with the relevant design guidance, and without the need for third party land.

## 4 Traffic Impact Assessment

### 4.1 Introduction

This section of the report sets out the methodology used for the calculation of the traffic flows used in the traffic modelling. A summary of the junction capacity assessments undertaken to establish the predicted future operation of the site access points and other junctions immediately adjacent to the site, is also provided.

### 4.2 Baseline Traffic Flows

The background traffic flow data for the three adjacent junctions has been growthed to account for potential development growth on the network. A 10\% increase in traffic has been assumed, this is considered to represent a worst case position as TEMPRO local growth suggests only an increase of $6.6 \%$ between 2010 and 2020.

The development will be accessed from at least two points on the local highway network. Principally, access will be gained from a modified A448 junction with Birchfield Road (Junction 4). Secondary access will be gained from Curr Lane, via a new link to the modified A448 junction with Birchfield Road.

As a result of the new junctions, baseline flows have been redistributed to account for the new junction with the A448 and the link road through the development. It is considered that at least $50 \%$ of the existing traffic on Foxlydiate will reassign via the new link road.

The growth and reassignment of traffic has resulted in revised traffic flows, which provide a robust baseline for modelling purposes.

Table 4.1 outlines the revised baseline traffic flows for all three original junctions, as well as the new A448 junction with the development. Appendix A provides full AM and PM peak traffic flow calculations and outlines the assumptions used.

Table 4.1: Baseline traffic flows (AM peak)

| Road | Survey <br> Flow | Baseline <br> Flow |  |
| :--- | ---: | ---: | :---: |
| Junction One | 167 | 184 |  |
| Birchfield Road (to A448 W'bound) | 364 | 360 |  |
| Birchfield Road | 581 | 642 |  |
| Birchfield Road (to A448 E'bound) | $\mathbf{1 1 1 2}$ | $\mathbf{1 1 8 6}$ |  |
| Total |  |  |  |
| Junction Two | 272 | 299 |  |
| Birchfield Road (East) | 147 | 122 |  |
| Foxlydiate Lane | 284 | 278 |  |
| Birchfield Road (West) | 703 | $\mathbf{6 9 9}$ |  |
| Total |  |  |  |


| Road | Survey Flow | Baseline Flow |
| :---: | :---: | :---: |
| Junction Three |  |  |
| Foxlydiate Lane | 86 | 61 |
| Church Road | 101 | 111 |
| Great Hockings Lane | 81 | 89 |
| Curr Lane | 32 | 69 |
| Total | 300 | 330 |
| Junction Four (new junction) |  |  |
| Birchfield Road (to A448 E/bound) | 581 | 656 |
| Birchfield Road (to A448 W/bound) | 383 | 438 |
| Development Link Road | 0 | 40 |
| Development Access | 0 | 0 |
| Total | 964 | 1135 |

### 4.3 Development Trip Generation

In order to calculate the vehicular demand from the development site, residential trip rates were calculated using the TRICS database. Appropriate selection parameters were used to ensure the trip rates were comparable for the type of development proposed in terms of its location. Assumptions included:

- Sites located outside of London;
- Sites located on the edge of town; and
- Residential houses - privately owned.

This selection provided eleven sites for calculations, average AM and PM peak trip rates were extracted and these are summarised alongside the trip generation in Table 4.2 below. Full TRICS outputs are contained within Appendix E.

Table 4.2: Trip rates and vehicle trips for the development

|  | AM |  | PM |  |
| :--- | ---: | ---: | ---: | ---: |
|  | Arrivals |  | Departures | Arrivals |
| Trip Rate | 0.153 | 0.465 | 0.44 | 0.256 |
| Vehicle Trips | 214 | 651 | 616 | 358 |
| Total Vehicle Trips | 865 |  | 974 |  |

### 4.4 Development Traffic: Trip Distribution

The development will be accessed from at least two points on the local highway network; the modified A448 junction with Birchfield Road and from Curr Lane, via a new link running through the development to the modified A448 junction with Birchfield Road.

Journey to Work data (Census 2001) for the surrounding residential areas provides the destinations of commuting trips for local residents. This data has been used to establish the distribution of development traffic. Table 4.3 provides a summary of the distribution, Appendix F contains the full Journey to Work data.

Table 4.3: Distribution of development trips

| Destination | Percentage Distribution |
| :--- | :--- |
| Redditch (North, includes town centre) | $33 \%$ |
| Redditch (South, includes hospital) | $19 \%$ |
| Birmingham and North | $19 \%$ |
| M5 North | $4 \%$ |
| Bromsgrove, Worcester and South West | $12 \%$ |
| M40, M1 and South | $13 \%$ |

Figure 4.1 illustrates the assignment assumptions, which feed into the traffic flow calculation sheets (Appendix A).


Figure 4.1: Distribution of development trips
Within the development it has been assumed that $30 \%$ of the development will be accessed directly from the fourth arm of the modified A448 junction, with the remaining 70\% accessed via the link road (A448 to Curr Lane).

In order to be able to assess if the new A448 junction, as well as the existing junctions will accommodate the baseline and development traffic, traffic flows have been calculated in line with methodology outlined in this section. A summary of these flows can be found in Table 4.4 below, with further detailed calculations in Appendix A.

Table 4.4: Baseline plus development traffic flows

| Road | Survey Flow | Baseline Flow | Baseline + Dev Flows |
| :---: | :---: | :---: | :---: |
| Junction One |  |  |  |
| Birchfield Road (to A448 W'bound) | 167 | 184 | 276 |
| Birchfield Road | 364 | 360 | 398 |
| Birchfield Road (to A448 E'bound) | 581 | 642 | 860 |
| Total | 1112 | 1186 | 1534 |
| Junction Two |  |  |  |
| Birchfield Road (East) | 272 | 299 | 343 |
| Foxlydiate Lane | 147 | 122 | 141 |
| Birchfield Road (West) | 284 | 278 | 392 |
| Total | 703 | 699 | 876 |
| Junction Three |  |  |  |
| Foxlydiate Lane | 86 | 61 | 67 |
| Church Road | 101 | 111 | 131 |
| Great Hockings Lane | 81 | 89 | 89 |
| Curr Lane | 32 | 69 | 151 |
| Total | 300 | 330 | 438 |
| Junction Four (new junction) |  |  |  |
| Birchfield Road (to A448 E/bound) | 581 | 656 | 714 |
| Birchfield Road (to A448 W/bound) | 383 | 438 | 568 |
| Development Link Road | 0 | 40 | 447 |
| Development Access | 0 | 0 | 195 |
| Total | 964 | 1135 | 1924 |

### 4.5 Junction Capacity Assessments

Junction capacity assessments have been undertaken in order to demonstrate that the existing highway network and proposed site access points, will operate with reserve capacity in the future with the inclusion of development traffic. The following junctions have been assessed:

- Junction 1: A448 Slips/Birchfield Road T-Junction
- Junction 2: Birchfield Road/Foxlydiate Road T Junction
- Junction 3: Foxlydiate Lane/Church Road/Great Hockings Lane/Curr Lane Roundabout
- Junction 4: Primary Site Access/Birchfield Road

A summary of the junction capacity assessments is provided overleaf and full modelling outputs are provided within Appendix G.

Table 4.5: Junction One (PICADY output)

| Junction One | AM |  |  | PM |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
|  | RFC | Delay | Queue | RFC | Delay | Queue |
| Birchfield Road to <br> Overbridge | 0.657 | 0.27 | 2 | 0.645 | 2 | 2 |
| Birchfield Road to <br> A448 | 0.710 | 0.48 | 2 | 0.423 | 1 | 1 |
| A448 right turn <br> into Birchfield <br> Road | 0.838 | 0.40 | 5 | 0.870 | 6 | 6 |

Key
RFC $=$ Ratio of flow to capacity (approaches typically considered to be at capacity at an RFC of 0.85 or $85 \%$ of capacity);
Delay $=$ Vehicle delay in seconds
Queue = Queues generated in Passenger Car Units (PCU's)
The PICADY results indicate that the existing A448/Birchfield Road priority junction would operate with reserve capacity and acceptable levels of queuing and delays in both peak periods; with the inclusion of development traffic and an additional $10 \%$ growth in background traffic to represent other developments which may come forward in the area.

Table 4.6: Junction Two (PICADY output)

| Junction Two | AM |  |  | PM |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
|  | RFC | Delay | Queue | RFC | Delay | Queue |
| Foxlydiate Lane | 0.273 | 0.19 | 0 | 0.406 | 0.22 | 1 |
| Birchfield Road <br> (West) | 0.211 | 0.17 | 1 | 0.072 | 0.13 | 0 |

Key
RFC $=$ Ratio of flow to capacity (approaches typically considered to be at capacity at an RFC of 0.85 or $85 \%$ of capacity);
Delay $=$ Vehicle delay in seconds
Queue $=$ Queues generated in Passenger Car Units (PCU's)

The PICADY results show that the existing Birchfield Road/Foxlydiate Lane priority junction would operate with high levels of reserve capacity and minimal queuing and delay in both peak periods; with the inclusion of development traffic and additional background growth on the network.

Table 4.7: Junction Three (ARCADY output)

| Junction Three | AM |  |  | PM |  |  |
| :--- | :---: | ---: | ---: | ---: | ---: | ---: |
|  | RFC | Delay |  | Queue | RFC | Delay |
| Queue |  |  |  |  |  |  |
| Foxlydiate Lane | 0.091 | 0.08 | 0 | 0.148 | 0.08 | 0 |
| Church Road | 0.153 | 0.07 | 0 | 0.190 | 0.08 | 0 |
| Great Hockings <br> Lane | 0.094 | 0.06 | 0 | 0.043 | 0.06 | 0 |
| Curr Lane | 0.193 | 0.08 | 0 | 0.169 | 0.08 | 0 |

Key
RFC $=$ Ratio of flow to capacity (approaches typically considered to be at capacity at an RFC of 0.85 or $85 \%$ of capacity);
Delay $=$ Vehicle delay in seconds
Queue = Queues generated in Passenger Car Units (PCU's)

ARCADY modelling has been undertaken to understand the impact of providing the new residential distributor road linking the modified A448 junction to Curr Lane, in relation to the existing Foxlydiate Lane/Church Road/Great Hockings Lane/Curr Lane roundabout. The results of the modelling indicate that the roundabout would operate with significant levels of reserve capacity in both peak periods and minimal queuing and delays, with the inclusion of development traffic and additional background growth on the network.

Table 4.8: Junction Four (ARCADY output)

| Junction Four |  | AM |  |  | PM |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | RFC | Delay | Queue | RFC | Delay | Queue |
| Birchfield Road (to A448 E/bound) | 0.733 | 0.15 | 3 | 0.688 | 0.13 | 2 |
| Birchfield Road (to A448 W/bound) | 0.578 | 0.11 | 1 | 0.820 | 0.20 | 4 |
| Development Link Road | 0.582 | 0.13 | 1 | 0.367 | 0.10 | 1 |
| Development <br> Access | 0.310 | 0.10 | 0 | 0.141 | 0.08 | 0 |
| Key |  |  |  |  |  |  |
| RFC $=$ Ratio of flow to capacity (approaches typically considered to be at capacity at an RFC of 0.85 or $85 \%$ of capacity); |  |  |  |  |  |  |
| Delay $=$ Vehicle delay in seconds |  |  |  |  |  |  |
| Queue = Queues generated in Passenger Car Units (PCU's) |  |  |  |  |  |  |

ARCADY modelling has been undertaken to reflect the proposed access strategy, which would see the current A448 junction being upgraded to provide a new fourarm roundabout and primary access to the site as shown in drawing no: CTB-AOE0003. The modelling results indicate that the proposed junction will provide sufficient levels of reserve capacity to accommodate the development proposals and additional background growth.

### 4.6 Summary

In summary, the amount of traffic predicted to use the local road network will not be significant, and most traffic will use the strategic regional routes. It is envisaged that the site will not create any congestion, or exacerbate existing problems on the local road network.

Junction capacity assessments have been undertaken for the site access junctions and other junctions immediately adjacent to the site. The traffic flows used in these assessments include development traffic for the Foxlydiate development and an allowance for additional growth on the network that may come forward as part of future development proposals in the local area.

The results of the junction capacity assessments clearly indicate that all junctions would operate with reserve capacity and minimal levels of queuing and congestion. It is therefore clear that the proposed access junctions would provide sufficient capacity to accommodate the development proposals and the potential redistribution of background traffic. It has also been demonstrated that the two off-site junctions linking to Foxlydiate Lane would also operate with capacity during the peak periods.

## 5 Summary

### 5.1 Current Network

The site is ideally located for walk journeys to schools and shops with walk routes generally pleasant, safe and lightly trafficked. The site will be designed to be permeable and will provide amenities on site to encourage shorter and more sustainable journeys.

The site is ideally located for cycle journeys to local schools, jobs, healthcare, colleges, Redditch town centre and the Railway Station. This will be significantly enhanced by providing connections into the important National Cycle Network, which passes the site (within 500 metres) and by having a site layout that is sustainable by design.

The location of the site provides access to healthcare, employment, retail, educational and leisure facilities within 30-60 minutes using a conventional bus. The site is within 400-800 metres local bus services.

The site benefits from its good connections to the Railway Station, which is only ten minutes away by cycle or by bus.

Current mode splits indicate that $75 \%$ of trips undertaken by residents in the surrounding area to the development are made by private vehicle (driver), with a further $6 \%$ commuting as car passengers. Therefore $19 \%$ of trips are undertaken by more sustainable modes of transport, such as bus, train, walking and cycling.

Journey to Work information indicates that 48\% of people work outside of Redditch town and $19 \%$ of people working in Redditch (south of the A448) most of whom are employed in areas around the Hospital, Park Farm and Washford. 33\% of people are shown to work in Redditch north of the A448, which includes Redditch town centre.

The local junctions surrounding the development site have also undergone an accident investigation (2005-2010 review period). Accident data shows a total of eleven reported accidents have taken place during the review period. When investigating the causation factors of these accidents, no accidents were caused by the same factor.

Currently there are no observed capacity issues regarding the three junctions surrounding the development site, with all junctions working within their capacity. Access to Redditch town centre and beyond is good with free flowing routes via the A441 Alvechurch Highway, Bromsgrove Road, and the B4184 Brockhill Drive.

### 5.2 Future Network

The proposed access vehicular access strategy for the site comprises two new access points and the provision of a link road between the A448 and Curr Lane.

The vehicular access strategy has been designed in accordance with the relevant design criteria and a Stage One Road safety Audit has also been undertaken. It has been demonstrated that access to the site can be provided which is safe, in accordance with the relevant design guidance, and without the need for third party land.

Junction capacity assessments have been undertaken for the site access junctions and other junctions immediately adjacent to the site. The traffic flows used in these assessments include development traffic for the Foxlydiate development and an allowance for additional growth on the network that may come forward as part of future development proposals in the local area.

The results of the junction capacity assessments clearly indicate that all junctions would operate with reserve capacity and minimal levels of queuing and congestion. It is therefore clear that the proposed access junctions would provide sufficient capacity to accommodate the development proposals and the potential redistribution of background traffic. It has also been demonstrated that the two off-site junctions linking to Foxlydiate Lane would also operate with capacity during the peak periods.

### 5.3 Key Considerations

From a transport perspective the key considerations to note are:

- The development is located on the south western side of Redditch which means that the site is fully accessible by sustainable modes to health, employment, retail and education facilities.
- The site benefits from being well positioned in relation to existing high quality strategic roads which are not subject to significant delays or congestion, therefore significant off-site highway improvements will not be required.
- The development provides an opportunity to improve bus services in the area. These improvements will benefit both existing residents of the area and future residents associated with this site and other development sites.
- The site presents an excellent opportunity to deliver significant growth without the need for major infrastructure scheme, thus the site will not be subject to the negative environmental impacts which can be associated with the implementation of such schemes.
- The site will provide a new vehicular link between the A448 and Curr Lane/Foxlydiate Lane, this presents a significant opportunity to relieve existing local traffic issues in Webheath.


### 5.4 Conclusion

From the evidence and analysis presented within this report, it is concluded that the Foxlydiate site offers an excellent opportunity to deliver growth on the edge of Redditch, without the need for major highway improvements to the wider network. The site also offers and opportunity to enhance sustainable links in the Webheath area, therefore befitting both existing and future residents of the area.

# Appendix A 

Traffic Flow Spreadsheets

## Appendix A Traffic Flow Calculation Spreadsheets

Includes:

- Current traffic flow spreadsheets
- Development traffic distribution
- Trip rate calculations




## Vehicle Trip Rates



TRICS 2010(b)v6.6.1 270610 B14.36 (16/9/2010)
Output saved under: U:ICTB AOE 000 - Foxlydiate Lane, Webheath, Redditch\Calcs\Traffic Surveys\Foxlydiate Trip Rates (TRICs).PDF

Dwelling assumption from Masterplan

Distribution (from JtW) - Vehicle Trips

| Origin/ Destination | \% | 8:00-09:00 |  | 17:00-18:00 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Arr | Dep | Arr | Dep |  |
| North Redditch | 33\% | 70 | 214 | 202 | 118 |  |
| South Redditch | 19\% | 41 | 124 | 117 | 68 |  |
| Birmingham and North | 19\% | 42 | 126 | 120 | 70 |  |
| M5 North | 4\% | 8 | 25 | 24 | 14 |  |
| Bromsgrove, Worcester and South West | 12\% | 25 | 76 | 72 | 42 |  |
| M40, M1 and South | 13\% | 28 | 86 | 81 | 47 |  |
|  |  | 214 | 651 | 616 | 358 |  |
|  | Check | 214 | 651 | 616 | 358 |  |
|  | 70\% | 150 | 456 | 431 | 251 | Development traffic off link road |
|  | 30\% | 64 | 195 | 185 | 108 | Development traffic off roundabout |

# Appendix B 

Road Safety - Accident Surveys

Appendix B Road Safety - Accident Survey
\(\left.\begin{array}{lll}Accidents between dates \quad \mathbf{0 1 / 0 8 / 2 0 0 5} \& and \& 31/07/2010 <br>

(6) months\end{array}\right]\)| Notes: |
| :--- |
| Selection: |
| Selected using Pre-defined Query: |


| Rec. | Reference | Severity | Date | Veh | Cas. | Time | Easting | Northing | Location |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 06DE85081 | Slight | 04/11/2006 | 2 | 1 | 0830 | 401740 | 267230 | BIRCHFIELD RD, REDDITCH J/W FOXLYDIATE LANE. |
| 2 | 08DE87674 | Slight | 12/01/2008 | 2 | 2 | 0700 | 401610 | 267490 | A448 BROMSGROVE HIGHWAY EB CW APPROX 160 M SE J/W BIRCHFIEL |
| 3 | 08DE87000 | Slight | 12/01/2008 | 1 | 1 | 0943 | 401640 | 267450 | A448, BROMSGROVE HIGHWAY, REDDITCH APPROX 90 M SE J/W BIRCH |
| 4 | 08DE87886 | Slight | 20/02/2008 | 2 | 6 | 0032 | 401490 | 267700 | B4096 HEWELL LANE R/ABOUT J/W B4184 BROMSGROVE |
| 5 | 08DE88073 | Slight | 18/03/2008 | 1 | 1 | 0815 | 401480 | 267700 | B4096 HEWELL LANE R/ABOUT J/W B4096 BIRCHFIELD RD REDDITCH |
| 6 | 08DE88179 | Slight | 06/04/2008 | 2 | 3 | 0755 | 401420 | 267710 | B4098, HEWELL RD TARDEBIGGE APPROX 65 MW J/W B 4184. |
| 7 | 09D900420 | Fatal | 20/01/2009 | 1 | 1 | 2335 | 401390 | 267680 | A448 BROMSGROVE HIGHWAY (LANE 2) 60 M W J/W B4096 |
| 8 | 09D901331 | Slight | 04/03/2009 | 1 | 1 | 1627 | 401495 | 267680 | A448 BROMSGROVE HIGHWAY, JW B4096 HEWELL LANE REDDITCH |
| 9 | 09D901825 | Slight | 26/03/2009 | 2 | 1 | 0930 | 401490 | 267700 | B4096 HEWELL LANE REDDITCH J/W B4184 ROUNDABOUT |
| 10 | 09D903929 | Slight | 17/07/2009 | 1 | 1 | 1915 | 401480 | 267630 | A448 BROMSGROVE HIGHWAY 35 M SE J/W BIRCHFIELD RD ISLAND |
| 11 | 09D904717 | Slight | 26/08/2009 | 2 | 1 | 1600 | 401490 | 267670 | A448 AT REDDITCH J/W B4096 HEWELL LANE ISLAND |
| 12 | 10D002108 | Serious | 22/04/2010 | 2 | 2 | 1436 | 401846 | 267165 | BIRCHFIELD ROAD, REDDITCH J/W REYNARD CLOSE |

Accidents between dates 01/08/2005 and 31/07/2010 (60) months
Selection:
Selected using Pre-defined Query:

Notes:
Foxlydiate Lane, Webheath


DR V2 BLINDED BY LOW SUN TURNING RT OFF BIRCHFIELD RD FAILS TO SEE V1 TRAVELLING IN OP DIRECTION.

Occurred on BIRCHFIELD RD, REDDITCH J/W FOXLYDIATE LANE.

| Vehicle Reference |  | Car |  | Going ahead other |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Vehicle movement from |  | SE to | NW | No tow / articulation |  |  |  |
|  |  |  |  | No skidding, jack-knifing or overturning |  |  |  |
| Location at impact | Jct Approach |  |  | First impact | Front |  | Hit vehicle: |
| Breath test |  | Negative |  |  | Age of Driver | 45 | Female |




Special Conditions at Site Mud
V1 LOSES CONTROL ON ICY RD SURFACE WHILST IN THE PROCESS OF OVERTAKING, V1 COMES TO A HALT IN CW AND IS STRUCK BY V2 WHICH IS UNABLE TO AVOID A COLLISION.

Occurred on A448 BROMSGROVE HIGHWAY EB CW APPROX 160 M SE J/W BIRCHFIELD RD SLIP ON RD.

| Vehicle Reference 1 | Car | Stopping |  |
| :--- | :---: | :---: | :--- |
| Vehicle movement from | NW to | SE | No tow / articulation |
|  |  |  | Skidded |


| Location at impact |  | Not at, or within 20M of Jct Negative |  |  |  | First impact |  | Back | Hit vehicle: |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Breath test |  |  |  |  |  | Age of Driver | 26 | Female |  |
| Casualty Ref: | 1 | Vehicle: | 1 | Age: | 26 | Female |  | Severity: | Slight |

Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Selected using Pre-defined Query :

Notes:
Foxlydiate Lane, Webheath
Vehicle Reference 2 Taxi/Private hire car Going ahead other Vehicle movement from NW to SE No tow / articulation Skidded

| Location at impact | Not at, or within 20 M of Jct | First impact | Front |
| :--- | :--- | :--- | :--- |
| Breath test | Negative | Age of Driver 23 | Female |

Casualty Ref: 2 Vehicle: 2 Age: 23 Female Driver/rider Severity: Slight


Special Conditions at Site None
DR V1 LOSES CONTROL ON ICY RD SURFACE, V1 LEAVES CW TO NS INTO TREES.

Occurred on A448, BROMSGROVE HIGHWAY, REDDITCH APPROX 90 M SE J/W BIRCHFIELD RD SLIP OFF RD.

| Vehicle Reference 1 |  | Car | Going ahead other |  |
| :--- | :--- | :---: | :--- | :--- |
| Vehicle movement from | SE | to | NW | No tow / articulation <br> Skidded and overturned |


| Location at impact | Not at, or within 20 M of Jct | First impact | Nearside |
| :--- | :--- | :--- | :--- |
| Breath test | Negative | Age of Driver 25 | Male |

Casualty Ref: 1 Vehicle: 1 Age: 25 Male Driver/rider Severity: Slight


Occurred on B4096 HEWELL LANE R/ABOUT J/W B4184 BROMSGROVE


Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Notes:
Selected using Pre-defined Query :

Foxlydiate Lane, Webheath


| 08DE88073 |  | $18 / 03 / 2008$ | Time | 0815 | Vehicles | 1 | Casualties | 1 |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Special Conditions at Site None
9 YR OLD CAS SAT IN TAXI AS PASS, NO SEAT BELTS FITTED IN TAXI, DRIVER APPARANTLY SINGING AND DANCING IN SEAT WHILE DRIVING, BRAKES SHARPLY ON APP/TO ISLAND CAUSING CAS TO HURT HER HEAD

Occurred on B4096 HEWELL LANE R/ABOUT J/W B4096 BIRCHFIELD RD REDDITCH

| Vehicle Reference 1 | Taxi/Private hire car Stopping |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Vehicle movement from | NW to | E | No tow/articulation |
|  |  |  | No skidding, jack-knifing or overturning |


| Location at impact | Jct Approach | First impact $\quad$ Did not impact | Hit vehicle: |
| :--- | :--- | :---: | :--- |
| Breath test | Driver not contacted | Age of Driver 28 | Male |

Casualty Ref: 1 Vehicle: 1 Age: 9 Female Passenger Severity: Slight


Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Selected using Pre-defined Query:

Notes:
Foxlydiate Lane, Webheath

Special Conditions at Site None
DR V1 LOSES CONTROL NEG LH BEND ON SNOW COVERED ICY RD SUR FACE, V1 CROSSES INTO PATH V2 TRAVELLING IN OP DIRECTION.

Occurred on B4098, HEWELL RD TARDEBIGGE APPROX 65 MW J/W B 4184.

| Vehicle Reference | 1 | Car |  |
| :--- | :---: | :---: | :--- |
| Gehicle movement from | NW to | E | No tow / articulation |
|  |  |  | Skidded and overturned |



| Location at impact | Not at, or within 20 M of Jct | First impact | Front |
| :--- | :--- | :--- | :--- |
| Breath test | Negative | Age of Driver 35 | Female |


| Casualty Ref: | 2 | Vehicle: 2 | Age: 35 | Female | Driver/rider | Severity: Slight |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Casualty Ref: | 3 | Vehicle: 2 | Age: 44 | Male | Passenger | Severity: Slight |



Occurred on A448 BROMSGROVE HIGHWAY (LANE 2) 60 M W J/W B4096

| Vehicle Reference 1 | Car | Going ahead other |  |
| :--- | :---: | :---: | :--- |
| Vehicle movement from | NW to | SE | No tow / articulation |
|  |  |  | Skidded |


| Location at impact | Not at, or within 20M of Jct Negative |  |  |  |  | First impact |  | Front | Hit vehicle: |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Breath test |  |  |  |  |  | Age of Driver | 31 | Female |  |
| Casualty Ref: | 1 | Vehicle: | 1 | Age: | 43 | Male | Pedestrian | Severity: | Fatal |

Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Selected using Pre-defined Query:

Notes:
Foxlydiate Lane, Webheath

| 09D901331 |  | 04/03/2009 | Time | 1627 | Vehicles |  | 1 | Casualties | 1 | Slight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| E: 401495 | N: | 267680 | First Road: | : A 448 | Roa | Type |  | Slip road |  |  |
| Speed limit: 30 | Junction Detail: Roundabout |  |  |  | Give way or controlled |  |  |  |  |  |
| Crossing: Control |  | ne |  | Facilities: | None within 5 |  |  |  |  | Dry |
| Daylight:street lights present |  |  |  |  | Fine without high winds |  |  |  |  |  |
| Special Conditions at Site None |  |  |  |  |  |  |  |  |  |  |
| DR V1 LOSES CONTROL ENTERING TRAFFIC ISLAND |  |  |  |  |  |  |  |  |  |  |

Occurred on A448 BROMSGROVE HIGHWAY, JW B4096 HEWELL LANE REDDITCH

| Vehicle Reference 1 <br> Vehicle movement from |  | Car |  | Going ahead left bend No tow / articulation Overturned |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NW to NE |  |  |  |  |  |  |  |
| Location at impact Breath test | Entering roundabout Not applicable |  |  | First impact |  | Front |  | Hit vehicle: Female |  |
| Casualty Ref: | 1 | Vehicle: | 1 | Age: | 25 | Female | Driver/rider | Severity: | Slight |



Occurred on B4096 HEWELL LANE REDDITCH J/W B4184 ROUNDABOUT


Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Selected using Pre-defined Query:

Notes:
Foxlydiate Lane, Webheath


Occurred on A448 BROMSGROVE HIGHWAY 35 M SE J/W BIRCHFIELD RD ISLAND

| Vehicle Reference 1 | Car | Going ahead left bend |  |
| :--- | :---: | :---: | :--- |
| Vehicle movement from | NE | to | S |$\quad$| No tow / articulation |
| :--- |
|  |


| Location at impact | Not at, or within 20 M of Jct | First impact | Front | Hit vehicle: |
| :--- | :--- | :--- | :--- | :--- |
| Breath test | Negative | Age of Driver | 18 | Female |

Casualty Ref: 1 Vehicle: 1 Age: 18 Female Driver/rider Severity: Slight

| 09D904717 | 26/08/2009 | Time | 1600 | Vehicles | 2 | Casualties | 1 | Slight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| E: 401490 | N: 267670 | First Road: | A 448 |  |  | 1 |  |  |
| Speed limit: 30 | Junction Detail: Roundabout |  |  | Give way or controlled |  |  |  |  |
| Crossing: Control | None |  | Facilities: | None within 5 |  |  | Road surface | Wet/Damp |
| Daylight:street lig | ts present |  |  |  |  | g without | winds |  |

Special Conditions at Site None

DR/V2 WAS STAT AT ISLAND WHEN STRUCK IN REAR BY V1

Occurred on A448 AT REDDITCH J/W B4096 HEWELL LANE ISLAND



Accidents between dates $\quad 01 / 08 / 2005$ and $\mathbf{3 1 / 0 7 / 2 0 1 0} \quad$ (60) months
Selection:
Notes:
Selected using Pre-defined Query :

Foxlydiate Lane, Webheath

Casualty Ref: 1 Vehicle: 2 Age: 43 Female Driver/rider Severity: Slight

| 10D002108 | 22/04/2010 | Time | 1436 | Vehicles | 2 | Casualties |  | Serious |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| E: 401846 | N: 267165 | First Road: | : U | Roa |  | Single carria |  |  |
| Speed limit: 30 | Junction Detail: T \& Stag Jct |  |  | Give way or controlled |  |  |  |  |
| Crossing: Control | None |  | Facilities: | None within 50 |  |  |  | Dry |

Daylight:street lights present
Fine without high winds

Special Conditions at Site None
VEH 1 AND VEH 2 TRAVELLING TOWARDS EACH OTHER ON THE OPPOS ITE SIDES OF THE CARRIAGEWAY. FOR AN UNKNOWN REASON VEH 1 HAS TRAVELLED ONTO THE OPPOSITE SIDE OF THE CARRIAGEWAY COLLIDING WITH VEH 2.

Occurred on BIRCHFIELD ROAD, REDDITCH J/W REYNARD CLOSE

| Vehicle Reference 1 | Car |  |  |  | Going ahead other |
| :--- | :---: | :---: | :--- | :---: | :---: |
| Vehicle movement from | SE | to | NW |  |  | | No tow / articulation |
| :--- |


Casualty Ref: 2 Vehicle: 2 Age: 47 Male Driver/rider Severity: Slight

## Accidents between dates

Selection:
Selected using Pre-defined Query :
$01 / 08 / 2005$ and $31 / 07 / 2010$

# Notes: 

Foxlydiate Lane, Webheath

Casualties:
Accidents involving:

| Motor vehicles only (excluding 2-wheels) | Fatal | Serious | Slight | Total |  | Fatal | Serious | Slight | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 1 | 10 | 12 | Vehicle driver | 0 | 1 | 12 | 13 |
|  |  |  |  |  | Passenger | 0 | 0 | 7 | 7 |
| 2-wheeled motor vehicles | 0 | 0 | 0 | 0 |  |  |  |  |  |
|  |  |  |  |  | Motorcycle rider | 0 | 0 | 0 | 0 |
|  |  |  |  |  | Cyclist | 0 | 0 | 0 | 0 |
| Pedal cycles | 0 | 0 | 0 | 0 |  |  |  |  |  |
|  |  |  |  |  | Pedestrian | 1 | 0 | 0 | 1 |
| Horses \& other | 0 | 0 | 0 | 0 | Other | 0 | 0 | 0 | 0 |
| Total | 1 | 1 | 10 | 12 | Total | 1 | 1 | 19 | 21 |



## Appendix C

Existing and proposed access drawings

## Appendix C <br> Existing and proposed access drawings

## Includes:

- CTBAOE001 - Existing Highway Network
- CTBAOE002 - Proposed Access Arrangements
- CTBAOE003 - Birchfield Road Access Arrangements
- CTBAOE004 - Curr Lane Access Arrangements






# Appendix D 

Road Safety Audit

## Appendix D Road Safety Audit

## Technical note

| Project | Heyford Developments, Land off <br> Foxlydiate Lane, Redditch | Date | 26 October 2010 |
| :--- | :--- | :---: | :--- |
| Note | Designers Response to Stage 1 Road <br> Safety Audit | Ref | CT BAOE/68 |
| Author | Alex Hayes |  |  |

## 1. Introduction

1.1 This Technical Note addresses the problems highlighted in the Stage 1 Road Safety Audit carried out by D Lines and J Richardson of Halcrow Safety Audit Section, Worcester, in October 2010.
1.2 All references made in this Note use the same referencing system as per the Audit.

## 2. Cross Section Variation

2.1 Problem A2.1.1 - Northern Roundabout safety fence
a) Problem accepted, provision for footway and safety fence to be incorporated into the design when upon embankment.

### 2.2 Problem A3.1.1- Northern roundabout severe entry deflections

a) Problem accepted, repositioning of approach alignments with inclusion of an intermediate radius to be provided.

### 2.3 Problem A3.1.2-Curr Lane T Junction, Shunt type accidents

a) Potential for accidents and inhibiting southbound through flow accepted; projected AADT flows into Curr Lane to be analysed in order to decide upon the form of junction to be used at this location. It is noted that due to the proximity to the existing roundabout, non standard tapers or Works at the tie in to the existing roundabout may be required in order to provide a ghost island right turn facility into Curr Lane.

# Appendix E 

TRICS assumptions and output

Appendix E TRICS assumptions and output

## TRIP RATE CALCULATI ON SELECTI ON PARAMETERS:

```
Land Use : 03-RESIDENTIAL
Category : A - HOUSES PRIVATELY OWNED
VEHI CLES
```

Selected regions and areas:
02 SOUTH EAST
EX ESSEX
04 EAST ANGLIA
CA CAMBRIDGESHIRE
SF SUFFOLK
05 EAST MI DLANDS
$\begin{array}{ll}\text { LN } & \text { LINCOLNSHIRE } \\ \text { NT } & \text { NOTTINGHAMSHIRE }\end{array}$
06 WEST MI DLANDS
ST STAFFORDSHIRE 1 days
WO WORCESTERSHIRE 2 days
08 NORTH WEST
LC LANCASHIRE
2 days
1 days

## Filtering Stage $\mathbf{2}$ selection:

| Parameter: | Number of dwellings |  |
| :--- | :--- | :--- |
| Range: | 48 to 237 (units: ) |  |
| Public Transport Provision: |  |  |
| Selection by: Include all surveys |  |  |

Date Range: $\quad 01 / 01 / 00$ to $26 / 06 / 09$
Selected survey days:

| Tuesday | 6 days |
| :--- | :--- |
| Wednesday | 1 days |
| Thursday | 3 days |
| Friday | 1 days |

Selected survey types:
Manual count
11 days
Directional ATC Count 0 days
Selected Locations:
Edge of Town
Selected Location Sub Categories:
Residential Zone
No Sub Category 3

LIST OF SITES relevant to selection parameters
$\begin{array}{lll}1 & \text { CA-03-A-01 } & \text { SEMI D./ TERRACED, CAMBR } \\ & \text { FALLOWFIELD } & \\ & \text { CHESTERTON } & \\ & \text { CAMBRIDGE } & \\ & \text { Edge of Town } & \\ & \text { Residential Zone } \\ & \text { Total Number of dwellings: } \\ \mathbf{2} & \text { CF-03-A-02 } & \text { MI XED HOUSES, CARDI FF }\end{array}$
DROPE ROAD
CARDIFF
Edge of Town
Residential Zone
Total Number of dwellings:
196
3 EX-03-A-01
MILTON ROAD
CORRINGHAM
STANFORD-LE-HOPE
Edge of Town
Residential Zone
Total Number of dwellings: 237
4 LC-03-A-22 BUNGALOWS, BLACKPOOL
CLIFTON DRIVE NORTH
BLACKPOOL
Edge of Town
Residential Zone
Total Number of dwellings:
98
5 LC-03-A-29
REVIDGE ROAD
FOUR LANE ENDS
BLACKBURN
Edge of Town
Residential Zone
Total Number of dwellings: 185
6 LN-03-A-01 MI XED HOUSES, LI NCOLN
BRANT ROAD
BRACEBRIDGE
LINCOLN
Edge of Town
Residential Zone
Total Number of dwellings: 150
7 NT-03-A-03 SEMI DETACHED,KI RKBY-I N-ASHFD NOTTI NGHAMSHI RE
B6018 SUTTON ROAD
KIRKBY-IN-ASHFIELD
Edge of Town
Residential Zone
Total Number of dwellings: 166
8 SF-03-A-02 SEMI DET./ TERRACED, I PSWICH
STOKE PARK DRIVE
MAIDENHALL
IPSWICH
Edge of Town
Residential Zone
Total Number of dwellings: 230
9 ST-03-A-03 MI XED HOUSES, STAFFORD
QUEENSVILLE
STAFFORD
Edge of Town
No Sub Category
Total Number of dwellings:
CAMBRI DGESHI RE

## CARDI FF

ESSEX

LANCASHI RE

LANCASHI RE

LI NCOLNSHIRE

E

LIST OF SITES relevant to selection parameters (Cont.)

| 10 WO-03-A-02 SEMI DETACHED, REDDITCH | WORCESTERSHI RE |  |  |
| :--- | :--- | :--- | :--- |
|  | MEADOWHILL ROAD |  |  |
|  |  |  |  |
|  | REDDITCH |  |  |
|  | Edge of Town |  |  |
|  | No Sub Category |  |  |
|  | Total Number of dwellings: | WORCESTERSHI RE |  |
| $\mathbf{1 1}$ | WO-03-A-06 TERRACED, BROMSGROVE |  |  |
|  | ST GODWALDS ROAD |  |  |
|  | ASTON FIELDS |  |  |
|  | BROMSGROVE |  |  |
|  | Edge of Town |  |  |
|  | No Sub Category |  |  |
|  | Total Number of dwellings: | 232 |  |

TRIP RATE for Land Use 03 - RESIDENTIAL/A - HOUSES PRIVATELY OWNED

## VEHI CLES

Calculation factor: 1 DWELLS BOLD print indicates peak (busiest) period


## Parameter summary

Trip rate parameter range selected:
48-237 (units: )
Survey date date range:
Number of weekdays (Monday-Friday):
01/01/00-26/06/09
Number of Saturdays:
11
Number of Sundays:
0
Surveys manually removed from selection:

# Appendix F 

Journey to Work data

## Appendix F Journey to Work data



| Ward | Zone | Tube/LRT | Train | Bus/Coach | Taxi | CarDrive | CarPax | MCycle | Bicycle | Walk | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Batchley | North Redditch | 0\% | 0\% | 10\% | 0\% | 51\% | 8\% | 1\% | 4\% | 26\% | 100\% |
|  | South Redditch | 0\% | 0\% | 26\% | 1\% | 52\% | 10\% | 3\% | 2\% | 5\% | 100\% |
|  | Birmingham and North | 0\% | 6\% | 1\% | 0\% | 83\% | 8\% | 1\% | 1\% | 1\% | 100\% |
|  | M5 North | 0\% | 3\% | 3\% | 0\% | 83\% | 12\% | 0\% | 0\% | 0\% | 100\% |
|  | Bromsgrove, Worcester and South West | 0\% | 0\% | 2\% | 1\% | 75\% | 8\% | 3\% | 2\% | 9\% | 100\% |
|  | M42, M1 and South | 0\% | 1\% | 4\% | 0\% | 85\% | 5\% | 3\% | 0\% | 2\% | 100\% |
| West | North Redditch | 0\% | 0\% | 10\% | 0\% | 73\% | 11\% | 0\% | 1\% | 4\% | 100\% |
|  | South Redditch | 0\% | 0\% | 4\% | 1\% | 80\% | 4\% | 1\% | 1\% | 10\% | 100\% |
|  | Birmingham and North | 0\% | 2\% | 2\% | 0\% | 92\% | 3\% | 1\% | 1\% | 0\% | 100\% |
|  | M5 North | 0\% | 0\% | 0\% | 0\% | 97\% | 3\% | 0\% | 0\% | 0\% | 100\% |
|  | Bromsgrove, Worcester and South West | 0\% | 0\% | 2\% | 0\% | 91\% | 4\% | 1\% | 0\% | 2\% | 100\% |
|  | M42, M1 and South | 1\% | 0\% | 1\% | 0\% | 94\% | 4\% | 0\% | 0\% | 0\% | 100\% |
| Total | North Redditch | 0\% | 0\% | 10\% | 0\% | 59\% | 9\% | 1\% | 3\% | 17\% | 100\% |
|  | South Redditch | 0\% | 0\% | 13\% | 1\% | 69\% | 7\% | 2\% | 2\% | 8\% | 100\% |
|  | Birmingham and North | 0\% | 4\% | 2\% | 0\% | 88\% | 5\% | 1\% | 1\% | 0\% | 100\% |
|  | M5 North | 0\% | 2\% | 2\% | 0\% | 89\% | 8\% | 0\% | 0\% | 0\% | 100\% |
|  | Bromsgrove, Worcester and South West | 0\% | 0\% | 2\% | 1\% | 84\% | 6\% | 2\% | 1\% | 5\% | 100\% |
|  | M40, M1 and South | 0\% | 0\% | 2\% | 0\% | 90\% | 5\% | 1\% | 0\% | 1\% | 100\% |

# Appendix G 

Modelling outputs

## Appendix G Modelling outputs

Includes:

- Junction One - PICADY results
- Junction Two - PICADY results
- Junction Three - ARCADY results
- Junction Four - ARCADY results

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

```
PICADY 4.1 ANALYSIS PROGRAM
RELEASE 4.0 (NOV 2003)
```

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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS IN NO WAY RELIEVED OF HIS RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"u: \CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch\Calcs \Traffic Surveys $\backslash m o d e l l i n g \backslash$ Final Modelling for Report\Junction 1 \Junction 1 AM Peak.vpi" (drive-on-the-left ) at 10:08:21 on Friday, 15 October 2010

## RUN TITLE

## ********

Junction 1: A448/Birchfield Road- AM Peak
.MAJOR/MINOR JUNCTION CAPACITY AND DELAY
$\star * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *$
INPUT DATA

MAJOR ROAD (ARM C) ---------------------- MAJOR ROAD (ARM A)
I
I
I
I
I
MINOR ROAD (ARM B)
ARM A IS A448 from offslip
ARM B IS Birchfield Road
ARM C IS A448 from Overbridge

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
ETC.

```
TRL
```

GEOMETRIC DATA

```
\begin{tabular}{|c|c|c|c|c|c|c|c|}
\hline I & \multicolumn{2}{|r|}{DATA ITEM} & I & MINOR & ROAD & & I \\
\hline I & \multicolumn{2}{|l|}{TOTAL MAJOR ROAD CARRIAGEWAY WIDTH} & I & ( W ) & 9.00 & M. & I \\
\hline I & \multicolumn{2}{|l|}{CENTRAL RESERVE WIDTH} & I & (WCR ) & 0.00 & M. & I \\
\hline I & & & I & & & & I \\
\hline I & \multirow[t]{4}{*}{MAJOR ROAD} & \multirow[t]{4}{*}{RIGHT TURN \(\begin{aligned} & - \text { WIDTH } \\ & - \text { VISIBILITY } \\ & - \text { BLOCKS TRAFFIC }\end{aligned}\)} & I & ( \(W C-B\) ) & 2.20 & M. & I \\
\hline I & & & I & ( \(\mathrm{VC}-\mathrm{B}\) ) & 30.0 & M. & I \\
\hline I & & & I & & NO & & I \\
\hline I & & & I & & & & I \\
\hline I & \multirow[t]{10}{*}{MINOR ROAD} & - VISIBILITY TO LEFT & I & (VB-C) & 30.0 & M. & I \\
\hline I & & - VISIBILITY TO RIGHT & I & (VB-A) & 30.0 & M. & I \\
\hline I & & - LANE 1 WIDTH & I & ( \(\mathrm{WB}-\mathrm{C}\) ) & - & & I \\
\hline I & & - LANE 2 WIDTH & I & (WB-A) & - & & I \\
\hline I & & - WIDTH AT 0 M FROM JUNC. & I & & 9.78 & M. & I \\
\hline I & & - WIDTH AT 5 M FROM JUNC. & I & & 4.78 & M. & I \\
\hline I & & - WIDTH AT 10 M FROM JUNC. & I & & 3.68 & M. & I \\
\hline I & & - WIDTH AT 15 M FROM JUNC. & I & & 3.66 & M. & I \\
\hline I & & - WIDTH AT 20 M FROM JUNC. & I & & 3.60 & M. & I \\
\hline I & & - LENGTH OF FLARED SECTION & I & & & VEHS & I \\
\hline
\end{tabular}

TRAFFIC DEMAND DATA

TIME PERIOD BEGINS 07.45 AND ENDS 09.15
```

LENGTH OF TIME PERIOD - 90 MINUTES.

```
LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|c|c|c|c|c|}
\hline I & & I & NUMBER OF & \multicolumn{4}{|l|}{MINUTES FROM START WHEN} & I & RATE & OF & FLOW ( & (VEH & /MIN) & I \\
\hline I & ARM & I & FLOW STARTS & I TOP & OF PEAK & I & FLOW STOPS & I & BEFORE & I & AT TOP & I & AFTER & I \\
\hline I & & I & TO RISE & I IS & REACHED & I & FALLING & I & PEAK & I & OF PEAK & K I & PEAK & I \\
\hline I & ARM A & I & 15.00 & I & 45.00 & I & 75.00 & I & 3.45 & I & 5.18 & I & 3.45 & I \\
\hline I & ARM B & I & 15.00 & I & 45.00 & I & 75.00 & I & 4.97 & I & 7.46 & I & 4.97 & I \\
\hline I & ARM C & I & 15.00 & I & 45.00 & I & 75.00 & I & 10.75 & I & 16.13 & I & 10.75 & I \\
\hline
\end{tabular}


TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline I
I
I & TIME & \[
\begin{array}{r}
\text { DEMAND } \\
\text { (VEH/MIN) }
\end{array}
\] & \[
\begin{aligned}
& \text { CAPACITY } \\
& \text { (VEH/MIN) }
\end{aligned}
\] & \[
\begin{gathered}
\text { DEMAND / } \\
\text { CAPACITY } \\
(\mathrm{RFC})
\end{gathered}
\] & PEDESTRIAN
FLOW
(PEDS/MIN) & START QUEUE (VEHS) & \[
\begin{gathered}
\text { END } \\
\text { QUEUE } \\
\text { (VEHS) }
\end{gathered}
\] & DELAY
(VEH.MIN/
TIME SEGMENT) & \(\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) }\end{array}\) \\
\hline I & \multicolumn{9}{|l|}{08.00-08.15} \\
\hline I & B-C & 4.12 & 9.07 & 0.454 & & 0.5 & 0.8 & 11.7 & I \\
\hline I & B-A & 1.82 & 4.66 & 0.391 & & 0.4 & 0.6 & 8.7 & I \\
\hline I & C-A & 6.91 & & & & & & & I \\
\hline I & C-B & 5.93 & 8.84 & 0.670 & & 1.2 & 1.9 & 26.5 & I \\
\hline I & A-B & 0.06 & & & & & & & I \\
\hline I & A-C & 4.06 & & & & & & & I \\
\hline I & & & & & & & & & I \\
\hline
\end{tabular}
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|c|}
\hline I
I
I & TIME & \begin{tabular}{l}
DEMAND \\
(VEH/MIN)
\end{tabular} & \[
\begin{aligned}
& \text { CAPACITY } \\
& \text { (VEH/MIN) }
\end{aligned}
\] & \[
\begin{gathered}
\text { DEMAND / } \\
\text { CAPACITY } \\
(R F C)
\end{gathered}
\] & PEDESTRIAN
FLOW
(PEDS/MIN) & START QUEUE (VEHS) & \[
\begin{gathered}
\text { END } \\
\text { QUEUE } \\
\text { (VEHS) }
\end{gathered}
\] & DELAY
(VEH.MIN/
TIME SEGMENT) & \begin{tabular}{l}
GEOMETRIC DELA \\
(VEH.MIN/ \\
TIME SEGMENT)
\end{tabular} & I
I
I \\
\hline I & \multicolumn{9}{|l|}{08.15-08.30} & I \\
\hline I & B-C & 5.05 & 7.25 & 0.696 & & 0.8 & 2.1 & 27.8 & & I \\
\hline I & B-A & 2.23 & 3.56 & 0.626 & & 0.6 & 1.5 & 19.6 & & I \\
\hline I & C-A & 8.46 & & & & & & & & I \\
\hline I & C-B & 7.26 & 8.66 & 0.838 & & 1.9 & 4.2 & 53.4 & & I \\
\hline I & A-B & 0.07 & & & & & & & & I \\
\hline I & A-C & 4.97 & & & & & & & & I \\
\hline I & & & & & & & & & & I \\
\hline
\end{tabular}
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline I
I
I & TIME & \[
\begin{array}{r}
\text { DEMAND } \\
\text { (VEH/MIN) }
\end{array}
\] & \[
\begin{aligned}
& \text { CAPACITY } \\
& \text { (VEH/MIN) }
\end{aligned}
\] & \[
\begin{gathered}
\text { DEMAND/ } \\
\text { CAPACITY } \\
\text { (RFC) }
\end{gathered}
\] & \[
\begin{gathered}
\text { PEDESTRIAN } \\
\text { FLOW } \\
\text { (PEDS/MIN) }
\end{gathered}
\] & \[
\begin{aligned}
& \text { START } \\
& \text { QUEUE }
\end{aligned}
\]
(VEHS) & \[
\begin{gathered}
\text { END } \\
\text { QUEUE } \\
\text { (VEHS) }
\end{gathered}
\] & DELAY
(VEH.MIN/
TIME SEGMENT) & \[
\begin{array}{cc}
\text { GEOMETRIC DELAYI } \\
\text { (VEH.MIN/ } & \text { I } \\
\text { TIME SEGMENT) } & \text { I }
\end{array}
\] \\
\hline I & \multicolumn{9}{|l|}{08.30-08.45} \\
\hline I & B-C & 5.05 & 7.68 & 0.657 & & 2.1 & 2.0 & 30.5 & I \\
\hline I & B-A & 2.23 & 3.14 & 0.710 & & 1.5 & 2.1 & 28.2 & I \\
\hline I & C-A & 8.46 & & & & & & & I \\
\hline I & C-B & 7.26 & 8.66 & 0.838 & & 4.2 & 4.6 & 66.6 & I \\
\hline I & A-B & 0.07 & & & & & & & I \\
\hline I & A-C & 4.97 & & & & & & & I \\
\hline I & & & & & & & & & I \\
\hline
\end{tabular}

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|c|}
\hline I & TIME & \begin{tabular}{l}
DEMAND \\
(VEH/MIN)
\end{tabular} & \[
\begin{aligned}
& \text { CAPACITY } \\
& \text { (VEH/MIN) }
\end{aligned}
\] & \[
\begin{gathered}
\text { DEMAND/ } \\
\text { CAPACITY } \\
(\mathrm{RFC})
\end{gathered}
\] & \[
\begin{gathered}
\text { PEDESTRIAN } \\
\text { FLOW } \\
\text { (PEDS/MIN) }
\end{gathered}
\] & \begin{tabular}{l}
START
QUEUE \\
(VEHS)
\end{tabular} & \[
\begin{gathered}
\text { END } \\
\text { QUEUE } \\
\text { (VEHS) }
\end{gathered}
\] & DELAY
(VEH.MIN/
TIME SEGMENT) & GEOMETRIC DELA (VEH.MIN/ TIME SEGMENT) & I \\
\hline I & \multicolumn{9}{|l|}{09.00-09.15} & I \\
\hline I & B-C & 3.45 & 9.79 & 0.352 & & 0.9 & 0.6 & 8.6 & & I \\
\hline I & B-A & 1.52 & 5.27 & 0.289 & & 0.7 & 0.4 & 6.6 & & I \\
\hline I & C-A & 5.79 & & & & & & & & I \\
\hline I & C-B & 4.96 & 8.98 & 0.553 & & 2.2 & 1.3 & 20.6 & & I \\
\hline I & A-B & 0.05 & & & & & & & & I \\
\hline I & A-C & 3.40 & & & & & & & & I \\
\hline I & & & & & & & & & & I \\
\hline
\end{tabular}
\begin{tabular}{ccc} 
QUEUE FOR STREAM & B-C \\
---------------------------- \\
TIME SEGMENT & NO. OF & \\
ENDING & VEHICLES & \\
& IN QUEUE & \\
08.00 & 0.5 & \(\star\) \\
08.15 & 0.8 & \(\star\) \\
08.30 & 2.1 & \(* *\) \\
08.45 & 2.0 & ** \\
09.00 & 0.9 & * \\
09.15 & 0.6 & \(*\)
\end{tabular}
QUEUE FOR STREAM B-A
------------------------
ENDING \(\quad\) NO. OF
VEHICLES
IN QUEUE
\begin{tabular}{ccc}
08.00 & 0.4 & \\
08.15 & 0.6 & \(*\) \\
08.30 & 1.5 & \(*\) \\
08.45 & 2.1 & \(* *\) \\
09.00 & 0.7 & \(*\) \\
09.15 & 0.4 &
\end{tabular}
\begin{tabular}{ccl} 
QUEUE FOR STREAM & C-B \\
Q------------------------- \\
TIME SEGMENT & NO. OF & \\
ENDING & VEHICLES & \\
& IN QUEUE & \\
08.00 & 1.2 & \(*\) \\
08.15 & 1.9 & \(* *\) \\
08.30 & 4.2 & \(* * *\) \\
08.45 & 4.6 & \(* * * *\) \\
09.00 & 2.2 & \(* *\) \\
09.15 & 1.3 & \(*\)
\end{tabular}

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD. * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS
```

PICADY 4.1 ANALYSIS PROGRAM
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Run with file:-
"u: \CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch \(\backslash\) Calcs \(\backslash\) Traffic Surveys \(\backslash m o d e l l i n g \backslash\) Final Modelling for Report\Junction 1 \Junction 1 PM Peak.vpi" (drive-on-the-left ) at 10:09:45 on Friday, 15 October 2010

\section*{RUN TITLE}
\(\star * * * * * * * *\)
Junction 1: A448/Birchfield Road- PM Peak
.MAJOR/MINOR JUNCTION CAPACITY AND DELAY
\(\star * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *\)
INPUT DATA

MAJOR ROAD (ARM C) ---------------------- MAJOR ROAD (ARM A)
I
I
I
I
I
MINOR ROAD (ARM B)
ARM A IS A448 from offslip
ARM B IS Birchfield Road
ARM C IS A448 from Overbridge

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
ETC.
```

TRL

```
GEOMETRIC DATA
```

| I | DATA ITEM |  | I | MINOR | ROAD B | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | TOTAL MAJOR ROAD CARRIAGEWAY WIDTH |  | I | ( W ) | 9.00 M. | I |
| I | CENTRAL RESERVE WIDTH |  | I | (WCR ) | 0.00 M . | I |
| I |  |  | I |  |  | I |
| I | MAJOR ROAD | RIGHT TURN - WIDTH | I | ( $\mathrm{WC}-\mathrm{B}$ ) | 2.20 M . | I |
| I |  | - VISIBILITY | I | ( $\mathrm{VC}-\mathrm{B}$ ) | 30.0 M . | I |
| I |  | - BLOCKS TRAFFIC | I |  | NO | I |
| I |  |  | I |  |  | I |
| I | MINOR ROAD | - VISIBILITY TO LEFT | I | (VB-C) | 30.0 M . | I |
| I |  | - VISIBILITY TO RIGHT | I | ( VB - A ) | 30.0 M . | I |
| I |  | - LANE 1 WIDTH | I | ( WB-C) | - | I |
| I |  | - LANE 2 WIDTH | I | ( WB-A) | - | I |
| I |  | - WIDTH AT 0 M FROM JUNC. | I |  | 9.78 M. | I |
| I |  | - WIDTH AT 5 M FROM JUNC. | I |  | 4.78 M . | I |
| I |  | - WIDTH AT 10 M FROM JUNC. | I |  | 3.68 M . | I |
| I |  | - WIDTH AT 15 M FROM JUNC. | I |  | 3.66 M . | I |
| I |  | - WIDTH AT 20 M FROM JUNC. | I |  | 3.60 M . | I |
| I |  | - LENGTH OF FLARED SECTION | I |  | 1 VEHS | I |

TRAFFIC DEMAND DATA

TIME PERIOD BEGINS 16.45 AND ENDS 18.15

```
LENGTH OF TIME PERIOD - 90 MINUTES.
```

LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

| I |  | I | NUM | MBER OF | MINUTES FROM START WHEN |  |  |  |  | I | RATE |  | FLOW ( | (VEH/MIN) |  | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | ARM | I | FLOW | STARTS | I | TOP | OF PEAK | I | FLOW STOPS | I | BEFORE | I | AT TOP | I | AFTER | I |
| I |  | I | TO | RISE | I | IS | REACHED | I | FALLING | I | PEAK | I | OF PEAK | I | PEAK | I |
| I | ARM A | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 6.05 | I | 9.08 | I | 6.05 | I |
| I | ARM B | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 4.40 | I | 6.60 | I | 4.40 | I |
| I | ARM C | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 8.07 | I | 12.11 | I | 8.07 | I |


| I |  | I |  | TURNING PROPORTIONS |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I |  | TURNING COUNTS (VEH/HR) |  |  |  |  |  |  |  |
| I |  | I |  | (PERCENTAGE OF H.V.S) |  |  |  |  |  |  |  |
| I |  |  |  |  |  |  |  |  |  |  |  |
| I | TIME | I | FROM/TO | I | ARM A I |  | ARM B I |  | ARM C I |  |  |
| I | $16.45-18.15$ | I |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM A | I | 0.000 | I | 0.037 | I | 0.9 | 963 | I |
| I |  | I |  | I | 0.0 | I | 18.0 | I | 466 | 6.0 | I |
| I |  | I |  | I | ( 0.0) | I | ( 2.0 ) | I | ( 2 | 2.0) |  |
| I |  | I |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM B | I | 0.199 | I | 0.000 | I | 0.8 | 801 | I |
| I |  | I |  | I | 70.0 | I | 0.0 | I | 282 | 2.0 |  |
| I |  | I |  | I | ( 2.0) | I | ( 0.0) | I | $(2$ | 2.0) |  |
| I |  | I |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM C | I | 0.418 | I | 0.582 | I | 0.0 | 000 | I |
| I |  | I |  | I | 270.0 | I | 376.0 | I |  | 0.0 | I |
| I |  | I |  | I | ( 2.0) | I | ( 2.0) | I | $(0$ | $0.0)$ | I |
| I |  | I |  | I |  | I |  | I |  |  | I |

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA




| I | TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | $\begin{aligned} & \text { CAPACITY } \\ & \text { (VEH/MIN) } \end{aligned}$ | $\begin{gathered} \text { DEMAND / } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | $\begin{gathered} \text { PEDESTRIAN } \\ \text { FLOW } \\ \text { (PEDS/MIN) } \end{gathered}$ | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc} \text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) } & \text { I } \end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 17.30-17.45 |  |  |  |  |  |  |  |  |
| I | B-C | 5.16 | 7.99 | 0.645 |  | 1.7 | 1.8 | 25.8 | I |
| I | B-A | 1.28 | 3.02 | 0.423 |  | 0.7 | 0.7 | 10.4 | I |
| I | C-A | 4.94 |  |  |  |  |  |  | I |
| I | C-B | 6.87 | 7.90 | 0.870 |  | 4.9 | 5.6 | 79.4 | I |
| I | A-B | 0.33 |  |  |  |  |  |  | I |
| I | A-C | 8.52 |  |  |  |  |  |  | I |
| I |  |  |  |  |  |  |  |  | I |



| I | TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | $\begin{aligned} & \text { CAPACITY } \\ & \text { (VEH/MIN) } \end{aligned}$ | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ (\mathrm{RFC}) \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | GEOMETRIC DELA <br> (VEH.MIN/ <br> TIME SEGMENT) | I I I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 18.00-18.15 |  |  |  |  |  |  |  |  |  |
| I | B-C | 3.53 | 9.56 | 0.369 |  | 0.9 | 0.6 | 9.3 |  | I |
| I | B-A | 0.88 | 4.91 | 0.178 |  | 0.3 | 0.2 | 3.5 |  | I |
| I | C-A | 3.38 |  |  |  |  |  |  |  | I |
| I | C-B | 4.70 | 8.46 | 0.556 |  | 2.3 | 1.3 | 21.0 |  | I |
| I | A-B | 0.22 |  |  |  |  |  |  |  | I |
| I | A-C | 5.82 |  |  |  |  |  |  |  | I |
| I |  |  |  |  |  |  |  |  |  | I |


| QUEUE FOR STREAM | B-C |  |
| :---: | :---: | :---: |
| QU------------------------- |  |  |
| TIME SEGMENT | NO. OF |  |
| ENDING | VEHICLES |  |
|  | IN QUEUE |  |
| 17.00 | 0.6 | $\star$ |
| 17.15 | 0.8 | $\star$ |
| 17.30 | 1.7 | $\star *$ |
| 17.45 | 1.8 | $\star *$ |
| 18.00 | 0.9 | $\star$ |
| 18.15 | 0.6 | $\star$ |

QUEUE FOR STREAM B-A
-------------------------
\(\left.\begin{array}{cc}IME SEGMENT \& NO. OF <br>
ENDING \& VEHICLES <br>

\& IN QUEUE\end{array}\right\}\)| 17.00 | 0.2 |
| :---: | :---: |
| 17.15 | 0.3 |
| 17.30 | 0.7 |
| 17.45 | 0.7 |
| 18.00 | 0.3 |
| 18.15 | 0.2 |

| QUEUE FOR STR | M C-B |  |
| :---: | :---: | :---: |
| TIME SEGMENT | NO. OF |  |
| ENDING | VEHICLES |  |
|  | IN QUEUE |  |
| 17.00 | 1.2 | * |
| 17.15 | 2.0 | ** |
| 17.30 | 4.9 | ** |
| 17.45 | 5.6 | ****** |
| 18.00 | 2.3 | ** |
| 18.15 | 1.3 | * |

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD


* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD. * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

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PICADY 4.1 ANALYSIS PROGRAM
RELEASE 4.0 (NOV 2003)
```

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Run with file:-
"u: \CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch Calcs \Traffic Surveys $\backslash m o d e l l i n g \backslash$
 (drive-on-the-left ) at 16:07:23 on Thursday, 14 October 2010

RUN TITLE

## *********

J2 - Birchfield Road/Foxlydiate Lane - AM Peak

```
.MAJOR/MINOR JUNCTION CAPACITY AND DELAY
```

$\star * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *$

```
INPUT DATA
```

MAJOR ROAD (ARM C) --------------------- MAJOR ROAD (ARM A)
I
$I$
$I$
$I$
I
I
MINOR ROAD (ARM B)
ARM A IS Birchfield Road (East)
ARM B IS Foxlydiate Lane
ARM C IS Birchfield Road (West)
STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
Etc.

## GEOMETRIC DATA

| I | DATA ITEM | I | MINO | ROAD | B | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | TOTAL MAJOR ROAD CARRIAGEWAY WIDTH | I | ( W ) | 6.97 |  | I |
| I | CENTRAL RESERVE WIDTH | I | (WCR ) | 0.00 |  | I |
| I |  | I |  |  |  | I |
| I | MAJOR ROAD RIGHT TURN - WIDTH | I | ( $\mathrm{WC}-\mathrm{B}$ ) | 2.20 |  | I |
| I | - VISIBILITY | I | ( $\mathrm{VC}-\mathrm{B}$ ) | 80.0 |  | I |
| I | - BLOCKS TRAFFIC | I |  | YES |  | I |
| I |  | I |  |  |  | I |
| I | MINOR ROAD - VISIBILITY TO LEFT | I | (VB-C) | 0.0 |  | I |
| I | - VISIBILITY TO RIGHT | I | (VB-A) | 0.0 |  | I |
| I | - LANE 1 WIDTH | I | ( $\mathrm{WB}-\mathrm{C}$ ) | 3.50 |  | I |
| I | - LANE 2 WIDTH | I | ( $\mathrm{WB}-\mathrm{A}$ ) | 0.00 |  | I |

TIME PERIOD BEGINS 07.45 AND ENDS 09.15
LENGTH OF TIME PERIOD - 90 MINUTES. LENGTH OF TIME SEGMENT - 15 MINUTES.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA


| I |  | I |  |  | TURNING PROPORTIONS |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I |  |  | TURNING COUNTS (VEH/HR) |  |  |  |  |  |  |  |
| I |  | I |  |  | (PERCENTAGE OF H.V.S) |  |  |  |  |  |  |  |
| I | TIME | I FROM/TO |  |  | I | ARM A I |  | ARM B I |  | ARM C I |  |  |
| I | 07.45-09.15 | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | A | I | 0.000 | I | 0.163 | I | 0.8 | 837 | I |
| I |  | I |  |  | I | 0.0 | I | 58.0 | I | 298 | 98.0 |  |
| I |  | I |  |  | I | ( 0.0) | I | ( 10.0) | I | ( 10 | 10.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | B | I | 0.495 | T | 0.000 | I | 0.5 | . 505 |  |
| I |  | I |  |  | I | 47.0 | I | 0.0 | I | 48 | 48.0 |  |
| I |  | I |  |  |  | ( 10.0) | I | ( 0.0) | I | ( 10 | 10.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | C | I | 0.817 | I | 0.183 | I | 0.0 | . 000 |  |
| I |  | I |  |  | I | 322.0 | I | 72.0 | I |  | 0.0 | I |
| I |  | I |  |  |  | ( 10.0) | I | ( 10.0) | I |  | 0.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
DEFAULT PROPORTIONS OF HEAVY VEHICLES ARE USED


TRL VIEWER 2.0 AE u: \.. \Junction $2 \backslash$ Junction 2 AM Peak.vpo - Page 3

| I I I | TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | START QUEUE (VEHS) | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) } & \text { I }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 08.15-08.30 |  |  |  |  |  |  |  |  |
| I | B-AC | 1.74 | 6.36 | 0.273 |  | 0.3 | 0.4 | 5.3 | I |
| I | C-AB | 2.55 | 12.05 | 0.211 |  | 0.3 | 0.5 | 6.9 | I |
| I | C-A | 4.66 |  |  |  |  |  |  | I |
| I | A-B | 1.06 |  |  |  |  |  |  | I |
| I | A-C | 5.45 |  |  |  |  |  |  | I |
| I |  |  |  |  |  |  |  |  | I |


| I I I | TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND / } \\ \text { CAPACITY } \\ (R F C) \end{gathered}$ | $\begin{gathered} \text { PEDESTRIAN } \\ \text { FLOW } \\ \text { (PEDS/MIN) } \end{gathered}$ | START QUEUE <br> (VEHS) | END QUEUE <br> (VEHS) | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) } & \text { I }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 08.30-08.45 |  |  |  |  |  |  |  | I |
| I | B-AC | 1.74 | 6.36 | 0.273 |  | 0.4 | 0.4 | 5.6 | I |
| I | C-AB | 2.55 | 12.06 | 0.211 |  | 0.5 | 0.5 | 7.0 | I |
| I | C-A | 4.65 |  |  |  |  |  |  | I |
| I | A-B | 1.06 |  |  |  |  |  |  | I |
| I | A-C | 5.45 |  |  |  |  |  |  | I |
| I |  |  |  |  |  |  |  |  | I |


| TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | CAPACITY <br> (VEH/MIN) | DEMAND/ CAPACITY (RFC) | PEDESTRIAN FLOW (PEDS/MIN) | START QUEUE (VEHS) | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) } & \text { I }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 08.45-09.00 |  |  |  |  |  |  |  |  |
| B-AC | 1.42 | 6.79 | 0.209 |  | 0.4 | 0.3 | 4.2 | I |
| C-AB | 1.84 | 11.54 | 0.160 |  | 0.5 | 0.3 | 4.7 | I |
| C-A | 4.04 |  |  |  |  |  |  | I |
| A-B | 0.87 |  |  |  |  |  |  | I |
| A-C | 4.45 |  |  |  |  |  |  | I |


| TIME | DEMAND (VEH/MIN) | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 09.00-09.15 |  |  |  |  |  |  |  |  |
| B-AC | 1.19 | 7.10 | 0.167 |  | 0.3 | 0.2 | 3.1 | I |
| C-AB | 1.41 | 11.16 | 0.126 |  | 0.3 | 0.2 | 3.4 | I |
| C-A | 3.52 |  |  |  |  |  |  | I |
| A-B | 0.73 |  |  |  |  |  |  | I |
| A-C | 3.72 |  |  |  |  |  |  | I |

*WARNING* NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

| TIME SEGMENT | NO. OF |
| :---: | ---: |
| ENDING | VEHICLES |
|  | IN QUEUE |
| 08.00 | 0.2 |
| 08.15 | 0.3 |
| 08.30 | 0.4 |
| 08.45 | 0.4 |
| 09.00 | 0.3 |
| 09.15 | 0.2 |

QUEUE FOR STREAM C-AB

## TIME SEGMENT NO. OF

ENDING VEHICLES
08.00 IN QUEUE
$08.15 \quad 0.2$
$\begin{array}{ll}08.15 & 0.3 \\ 08.30 & 0.5\end{array}$
$08.45 \quad 0.5$
$09.00 \quad 0.3$
$09.15 \quad 0.2$

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TRL TRL VIEWER 2.0 AE u:\.. \Junction 2\Junction 2 AM Peak.vpo - Page 4
```

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD


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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

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PICADY 4.1 ANALYSIS PROGRAM
RELEASE 4.0 (NOV 2003)
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Run with file:-
"u: \CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch Calcs \Traffic Surveys $\backslash m o d e l l i n g \backslash$
 (drive-on-the-left ) at 16:10:26 on Thursday, 14 October 2010

RUN TITLE

## *********

J2 - Birchfield Road/Foxlydiate Lane - PM Peak

```
.MAJOR/MINOR JUNCTION CAPACITY AND DELAY
```

$\star * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *$

```
INPUT DATA
```

MAJOR ROAD (ARM C) --------------------- MAJOR ROAD (ARM A)
I
$I$
$I$
$I$
I
I
MINOR ROAD (ARM B)
ARM A IS Birchfield Road (East)
ARM B IS Foxlydiate Lane
ARM C IS Birchfield Road (West)
STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
Etc.

## GEOMETRIC DATA

| I | DATA ITEM | I | MINO | ROAD | B | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | TOTAL MAJOR ROAD CARRIAGEWAY WIDTH | I | ( W ) | 6.97 |  | I |
| I | CENTRAL RESERVE WIDTH | I | (WCR ) | 0.00 |  | I |
| I |  | I |  |  |  | I |
| I | MAJOR ROAD RIGHT TURN - WIDTH | I | ( $\mathrm{WC}-\mathrm{B}$ ) | 2.20 |  | I |
| I | - VISIBILITY | I | ( $\mathrm{VC}-\mathrm{B}$ ) | 80.0 |  | I |
| I | - BLOCKS TRAFFIC | I |  | YES |  | I |
| I |  | I |  |  |  | I |
| I | MINOR ROAD - VISIBILITY TO LEFT | I | (VB-C) | 0.0 |  | I |
| I | - VISIBILITY TO RIGHT | I | (VB-A) | 0.0 |  | I |
| I | - LANE 1 WIDTH | I | ( $\mathrm{WB}-\mathrm{C}$ ) | 3.50 |  | I |
| I | - LANE 2 WIDTH | I | ( $\mathrm{WB}-\mathrm{A}$ ) | 0.00 |  | I |

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

| I | ARM | I | NUMBER OF |  | MINUTES FROM START WHEN |  |  |  |  | I | RATE | OF FLOW |  |  | (VEH/MIN) |  | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I | FLOW | STARTS | I | TOP | OF PEAK | I | FLOW STOPS | I | BEFORE | I | AT | TOP | I | AFTER | I |
| I |  | I | TO | RISE | I | IS | REACHED | I | FALLING | I | PEAK | I | OF | PEAK | I | PEAK | I |
| I | ARM A | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 4.29 | I |  | 6.43 | I | 4.29 | I |
| I | ARM B | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 1.77 | I |  | 2.66 | I | 1.77 |  |
| I | ARM C | I |  | 15.00 | I |  | 45.00 | I | 75.00 | I | 4.90 | I |  | 7.35 | I | 4.90 |  |


| I |  | I |  |  | TURNING PROPORTIONS |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I |  |  | TURNING COUNTS (VEH/HR) |  |  |  |  |  |  |  |
| I |  | I |  |  | (PERCENTAGE OF H.V.S) |  |  |  |  |  |  |  |
| I | TIME | I FROM/TO |  |  | I | ARM A I |  | ARM B I |  | ARM C I |  |  |
| I | 16.45-18.15 | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | A | I | 0.000 | I | 0.114 | I | 0.8 | . 886 |  |
| I |  | I |  |  | I | 0.0 | I | 39.0 | I | 304 | 4.0 |  |
| I |  | I |  |  | I | ( 0.0) | I | ( 10.0) | I | ( 10 | 10.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | B | I | 0.528 | T | 0.000 | I | 0.4 | . 472 |  |
| I |  | I |  |  | I | 75.0 | I | 0.0 | I | 67 | 67.0 |  |
| I |  | I |  |  |  | ( 10.0) | I | ( 0.0) | I | ( 10 | 10.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |
| I |  | I | ARM | C | I | 0.939 | I | 0.061 | I |  | . 000 |  |
| I |  | I |  |  | I | 368.0 | I | 24.0 | I |  | 0.0 | I |
| I |  | I |  |  |  | ( 10.0) | I | ( 10.0) | I |  | 0.0) |  |
| I |  | I |  |  | I |  | I |  | I |  |  | I |

TURNING PROPORTIONS ARE CALCULATED FROM TURNING COUNT DATA
DEFAULT PROPORTIONS OF HEAVY VEHICLES ARE USED

| I I I | TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | $\begin{aligned} & \text { CAPACITY } \\ & \text { (VEH/MIN) } \end{aligned}$ | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ (\mathrm{RFC}) \end{gathered}$ | $\begin{gathered} \text { PEDESTRIAN } \\ \text { FLOW } \\ (\text { PEDS/MIN) } \end{gathered}$ | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | GEOMETRIC DELA (VEH.MIN/ TIME SEGMENT) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 16.45-1 | . 00 |  |  |  |  |  |  |  |
| I | B-AC | 1.78 | 7.09 | 0.250 |  | 0.0 | 0.3 | 4.7 |  |
| I | $C-A B$ | 0.48 | 11.52 | 0.042 |  | 0.0 | 0.1 | 0.9 |  |
| I | C-A | 4.42 |  |  |  |  |  |  |  |
| I | A-B | 0.49 |  |  |  |  |  |  |  |
| I | A-C | 3.80 |  |  |  |  |  |  |  |
| I |  |  |  |  |  |  |  |  |  |


| TIME | DEMAND (VEH/MIN) | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | START QUEUE (VEHS) | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | GEOMETRIC DELA <br> (VEH.MIN/ <br> TIME SEGMENT) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17.00-17.15 0.80 ${ }^{\text {c }}$ |  |  |  |  |  |  |  |  |
| B-AC | 2.12 | 6.80 | 0.312 |  | 0.3 | 0.4 | 6.4 |  |
| C-AB | 0.63 | 11.93 | 0.053 |  | 0.1 | 0.1 | 1.2 |  |
| C-A | 5.22 |  |  |  |  |  |  |  |
| A-B | 0.58 |  |  |  |  |  |  |  |
| A-C | 4.54 |  |  |  |  |  |  |  |

TRL VIEWER 2.0 AE u:\.. \Junction $2 \backslash$ Junction 2 PM Peak.vpo - Page 3

| I I I | TIME | DEMAND (VEH/MIN) | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | START QUEUE (VEHS) | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | GEOMETRIC DELAYI <br> (VEH.MIN/ I <br> TIME SEGMENT) I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 17.15-17.30 |  |  |  |  |  |  |  |  |
| I | B-AC | 2.60 | 6.40 | 0.406 |  | 0.4 | 0.7 | 9.5 | I |
| I | C-AB | 0.92 | 12.67 | 0.072 |  | 0.1 | 0.1 | 1.9 | I |
| I | C-A | 6.25 |  |  |  |  |  |  | I |
| I | A-B | 0.71 |  |  |  |  |  |  | I |
| I | A-C | 5.56 |  |  |  |  |  |  | I |
| I |  |  |  |  |  |  |  |  | I |



| TIME | $\begin{array}{r} \text { DEMAND } \\ \text { (VEH/MIN) } \end{array}$ | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ (\text { RFC }) \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | START QUEUE (VEHS) | END QUEUE (VEHS) | DELAY <br> (VEH.MIN/ <br> TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) } & \text { I }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17.45-18.00 0.80 |  |  |  |  |  |  |  |  |
| B-AC | 2.12 | 6.80 | 0.312 |  | 0.7 | 0.5 | 7.2 | I |
| C-AB | 0.63 | 11.93 | 0.053 |  | 0.1 | 0.1 | 1.2 | I |
| C-A | 5.22 |  |  |  |  |  |  | I |
| A-B | 0.58 |  |  |  |  |  |  | I |
| A-C | 4.54 |  |  |  |  |  |  | I |


| TIME | DEMAND (VEH/MIN) | CAPACITY <br> (VEH/MIN) | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | PEDESTRIAN FLOW (PEDS/MIN) | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | DELAY (VEH.MIN/ TIME SEGMENT) | $\begin{array}{cc}\text { GEOMETRIC DELAYI } \\ \text { (VEH.MIN/ } & \text { I } \\ \text { TIME SEGMENT) }\end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 18.00-18.15 |  |  |  |  |  |  |  |  |
| B-AC | 1.78 | 7.09 | 0.250 |  | 0.5 | 0.3 | 5.3 | I |
| C-AB | 0.48 | 11.52 | 0.042 |  | 0.1 | 0.1 | 0.9 | I |
| C-A | 4.42 |  |  |  |  |  |  | I |
| A-B | 0.49 |  |  |  |  |  |  | I |
| A-C | 3.80 |  |  |  |  |  |  | I |

*WARNING* NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

| --------------------------1 |  |
| :---: | :---: |
| TIME SEGMENT | NO. OF |
| ENDING | VEHICLES |
|  | IN QUEUE |
| 17.00 | 0.3 |
| 17.15 | 0.4 |
| 17.30 | 0.7 |
| 17.45 | 0.7 |
| 18.00 | 0.5 |
| 18.15 | 0.3 |

QUEUE FOR STREAM C-AB
TIME SEGMENT NO. OF

ENDING VEHICLES
IN QUEUE

| 17.00 | 0.1 |
| :--- | :--- |
| 17.15 | 0.1 |

$17.30 \quad 0.1$
$17.45 \quad 0.1$
$18.00 \quad 0.1$
$18.15 \quad 0.1$

```
TRL TRL VIEWER 2.0 AE u:\.. \Junction 2\Junction 2 PM Peak.vpo - Page 4
```

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD


[^1]$\qquad$

## ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

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| :--- | :--- | :--- | :--- |
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| Nine Mile Ride | Email: softwarebureau@trl.co.uk |  |
| Wokingham, Berks. | Web: | www.trlsoftware.co.uk |
| RG40 3GA, UK |  |  |

THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

## Run with file:-

"u:\CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch\Calcs Traffic Surveys $\backslash m o d e l l i n g \backslash$
 (drive-on-the-left ) at 16:12:47 on Thursday, 14 October 2010

FILE PROPERTIES
**************

RUN TITLE: Junction 3: Secondary Site Access - AM Peak
LOCATION: Redditch
DATE: 17/09/2010
CLIENT: Heyford Developments
ENUMERATOR: ME
JOB NUMBER: CTBAOE
STATUS: Preliminary
DESCRIPTION:

```
INPUT DATA
```

ARM A - Foxlydiate Lane
ARM B - Church Road
ARM C - Great Hockings Lane
ARM D - Curr Lane

GEOMETRIC DATA

$\mathrm{V}=$ approach half-width
$\mathrm{L}=$ effective flare length
$R=$ entry radius
$\mathrm{E}=$ entry width

D = inscribed circle diameter
PHI = entry angle
(Only sets included in the current run are shown)
------

I ARM I FLOW SCALE (\%) I
I A I 100

| I | A | I | 100 | I |
| :--- | :--- | :--- | :--- | :--- |
| I | $B$ | I | 100 | I |
| I | C | I | 100 | I |

$\begin{array}{llll}\text { I D I } & 100 & I\end{array}$

```
LENGTH OF TIME PERIOD - 90 MINUTES.
LENGTH OF TIME SEGMENT - 15 MINUTES,
```

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

DEMAND SET TITLE: AM Peak


DEMAND SET TITLE: AM Peak


QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT



QUEUE AT ARM A

| TIME SEGMENT | NO. OF |
| :---: | :---: |
| ENDING |  | | VEHICLES |
| :---: | :---: |
| IN QUEUE |

QUEUE AT ARM B
TIME SEGMENT

ENDING | NO. OF |
| ---: |
| VEHICLES |
| IN QUEUE |\(~\left(\begin{array}{cc} <br>

08.00 \& 0.1 <br>
08.15 \& 0.1 <br>
08.30 \& 0.2 <br>
08.45 \& 0.2 <br>
09.00 \& 0.1 <br>
09.15 \& 0.1\end{array}\right.\)

QUEUE AT ARM C

TIME SEGMENT NO. OF
ENDING VEHICLES IN QUEUE

| 08.00 | 0.1 |
| :--- | :--- |
| 08.15 | 0.1 |
| 08.30 | 0.1 |
| 08.45 | 0.1 |
| 09.00 | 0.1 |
| 09.15 | 0.1 |

## QUEUE AT ARM D

| TIME SEGMENT | NO. OF |
| :---: | ---: |
| ENDING | VEHICLES |
|  | IN QUEUE |

## QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

| I | ARM | I | TOTAL DEMAND |  | I | * QUEUEING * |  |  | I | INCLUSIVE |  | QUEUEING * | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I |  |  | I | * D | DEL | AY | I | * DELAY * |  |  | I |
| I |  | I |  |  |  |  |  |  |  |  |  |  | I |
| I |  | I | (VEH) | (VEH/H) | I | (MIN) |  | (MIN/VEH) | I | (MIN) |  | (MIN/VEH) | I |
| I | A | I | 93.2 | I 62.2 | I | 7.2 | I | 0.08 | I | 7.2 | I | 0.08 | I |
| I | B | I | 179.6 I | I 119.8 | I | 13.0 | I | 0.07 | I | 13.0 | I | 0.07 | I |
| I | C | I | 123.4 | I 82.3 | I | 7.5 | I | 0.06 | I | 7.5 | I | 0.06 | I |
| I | D | I | 207.1 | I 138.0 | I | 17.0 | I | 0.08 | I | 17.0 | I | 0.08 | I |
| I | ALL | I | 603.3 I | I 402.2 | I | 44.8 |  | 0.07 | I | 44.8 | I | 0.07 | I |

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD. * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB
$\qquad$
ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

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| Nine Mile Ride | Email: | softwarebureau@trl.co.uk |
| Wokingham, Berks. | Web: | www.trlsoftware.co.uk |
| RG40 3GA, UK |  |  |

u: \CTB AOE 000 - Fox7ydiate Lane, Webheath, Redditch\Ca1cs\Traffic Surveys
Final Modelling for Report\Junction $3 \backslash$ Junction 3 secondary Site Access PM Peak.vai"
(drive-on-the-1eft ) at 14:13:33 on Thursday, 14 October 2010
FILE PROPERTIES
*********
RUN TITLE: Junction 3: Secondary Site Access - PM Peak
LOCATION: Redditch
DATE: $17 / 09 / 2010$
CLIENT: Heyford Developments
ENUMERATOR: ME
JOB NUMBER: CTBAOE
STATUS: Preliminary
DESCRIPTION:
INPUT DATA
※***
ARM A - Foxlydiate Lane
ARM B - Church Road
ARM C - Great Hockings Lane
ARM D - Curr Lane

GEOMETRIC DATA


V = approach half-width
$\mathrm{E}=$ entry width

## $L=e f f e c t i v e ~ f l a r e ~ 1 e n g t h ~$ <br> $\mathrm{R}=$ entry radius

$\mathrm{D}=$ inscribed circle diameter
PHI = entry angle
(Only sets included in the current run are shown)

| I ARM | I | FLOW | SCALE (\%) |
| :---: | :---: | :---: | :---: |
|  | I |  |  |
| I A | I | 100 | I |
| I | B | I | 100 |
| I C C | I | 100 | I |
| I | D | I | 100 |

DEMAND SET TITLE: PM Peak


DEMAND SET TITLE: PM Peak


QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT




## QUEUE AT ARM A

| TIME SEGMENT <br> ENDING | NO. OF <br> VEHICLES <br> IN QUEUE |
| :---: | :---: |
|  |  |
| 17.00 | 0.1 |
| 17.15 | 0.1 |
| 17.30 | 0.2 |
| 17.45 | 0.2 |
| 18.00 | 0.1 |
| 18.15 | 0.1 |

QUEUE AT ARM B

| TIME SEGMENT | NO. OF <br> ENDING |
| :---: | :---: |
|  | VEHICLES <br> IN QUEUE |
| 17.00 | 0.1 |
| 17.15 | 0.2 |
| 17.30 | 0.2 |
| 17.45 | 0.2 |
| 18.00 | 0.2 |
| 18.15 | 0.1 |

QUEUE AT ARM C

| TIME SEGMENT <br> ENDING | NO. OF <br> VEHICLES <br> IN QUEUE |
| :---: | ---: |
|  |  |
| 17.00 | 0.0 |
| 17.15 | 0.0 |
| 17.30 | 0.0 |
| 17.45 | 0.0 |
| 18.00 | 0.0 |
| 18.15 | 0.0 |

QUEUE AT ARM D

| TIME SEGMENT | NO. OF <br> ENDING |
| :--- | ---: |
|  | VEHICLES <br> IN QUEUE |
| 17.00 | 0.1 |
| 17.15 | 0.2 |
| 17.30 | 0.2 |
| 17.45 | 0.2 |
| 18.00 | 0.2 |
| 18.15 | 0.1 |



* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD. * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB
$\qquad$
$\qquad$ ARCAD Y 6 $\qquad$
ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"u:\CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch\Ca1cs\Traffic Surveys\mode11ing\}
 (drive-on-the-1eft ) at 13:59:03 on Thursday, 14 October 2010

FILE PROPERTIES
***************

RUN TITLE: Junction 4: Primary Site Access - AM Peak
LOCATION: Redditch
DATE: 17/09/2010
CLIENT: Heyford Developments
ENUMERATOR: ME
JOB NUMBER: CTBAOE
STATUS: Preliminary DESCRIPTION:

INPUT DATA
**********
ARM A - A448 from overbridge
ARM B - From offslip
ARM C - Link Road
ARM D - Site Access

GEOMETRIC DATA


V = approach half-width
$E=$ entry width

$$
\begin{aligned}
& \mathrm{L}=\text { effective flare length } \\
& \mathrm{R}=\text { entry radius }
\end{aligned}
$$

D = inscribed circle diameter
PHI = entry angle

| I ARM | I | FLOW | SCALE (\%) |
| :---: | :---: | :---: | :---: |
|  | I |  |  |
| I A | I | 100 | I |
| I | B | I | 100 |
| I C C | I | 100 | I |
| I | D | I | 100 |

DEMAND SET TITLE: AM Peak


DEMAND SET TITLE: AM Peak

| I |  | I | TURNING PROPORTIONS |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I | TURNING COUNTS (VEH/HR) |  |  |  |  |  |  |
| I |  | I |  |  |  | ERCENTAGE | OF H.V.S |  | I |
| I | TIME |  |  |  |  |  |  |  |  |
| I |  | I | FROM/TO |  | I | ARM A I | ARM B | I ARM C I | ARM D I |
| I | 07.45-09.15 | I |  |  | I | I |  | I |  |
| I |  | I | ARM | A | I | 0.000 I | 0.895 | I 0.081 I | 0.024 I |
| I |  | I |  |  | I | 0.0 I | 639.0 | I 58.0 I | 17.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0) | I ( 10.0) I | ( 10.0) I |
| I |  | I |  |  | I | I | - 0.000 | I I |  |
| I |  | I | ARM | B | I | 0.741 I | 0.000 | 0.190 I | 0.069 I |
| I |  | I |  |  | I | 421.0 I | 0.0 | I 108.0 I | 39.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0) | ( 10.0)I | ( 10.0) I |
| I |  | I |  |  | I | I | ( 0.387 | I I |  |
| I |  | I | ARM | C | I | 0.595 I | 0.387 | I 0.000 I | 0.018 I |
| I |  | I |  |  | I | 266.0 I | 173.0 | I 0.0 I | 8.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0) | I ( 10.0)I | ( 10.0) I |
| I |  | I |  |  | I |  | I | I I |  |
| I |  | I | ARM | D | I | 0.541 I | 0.335 | I 0.124 I | 0.000 I |
| I |  | I |  |  | I | 105.0 I | ( 65.0 I | I 24.0 I | 0.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0) | I ( 10.0)I | ( 10.0) I |
| I |  | I |  |  | I | I | - I | I I |  |

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT



| I I I | TIME | $\begin{aligned} & \text { DEMAND } \\ & \text { (VEH/MIN) } \end{aligned}$ | $\begin{aligned} & \text { CAPACITY } \\ & \text { (VEH/MIN) } \end{aligned}$ | $\begin{gathered} \text { DEMAND/ } \\ \text { CAPACITY } \\ \text { (RFC) } \end{gathered}$ | $\begin{gathered} \text { PEDESTRIAN } \\ \text { FLOW } \\ \text { (PEDS/MIN) } \end{gathered}$ | $\begin{gathered} \text { START } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { END } \\ \text { QUEUE } \\ \text { (VEHS) } \end{gathered}$ | $\begin{gathered} \text { DELAY } \\ \text { (VEH.MIN/ } \\ \text { TIME SEGMENT) } \end{gathered}$ | GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT) | AVERAGE DELAY <br> PER ARRIVING <br> VEHICLE (MIN) | $I$ $I$ $I$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I | 08.45 | . 00 |  |  |  |  |  |  |  |  | I |
| I | ARM A | 10.66 | 18.28 | 0.583 |  | 2.7 | 1.4 | 22.5 |  | 0.13 | I |
| I | ARM B | 8.48 | 18.13 | 0.468 |  | 1.4 | 0.9 | 13.8 |  | 0.10 | I |
| I | ARM C | 6.67 | 14.86 | 0.449 |  | 1.4 | 0.8 | 12.9 |  | 0.12 | I |
| I | ARM D | 2.90 | 12.99 | 0.223 |  | 0.4 | 0.3 | 4.5 |  | 0.10 | I |
| I |  |  |  |  |  |  |  |  |  |  | I |
| I | TIME |  |  |  | PEDESTRIAN | START | END | DELAY | GEOMETRIC DELAY | AVERAGE DELAY | I |
| I |  | (VEH/MIN) | (VEH/MIN) | CAPACITY | FLOW | QUEUE | QUEUE | (VEH.MIN/ | (VEH.MIN/ | PER ARRIVING | I |
| I |  |  |  | (RFC) | (PEDS/MIN) | (VEHS) | (VEHS) | TIME SEGMENT) | TIME SEGMENT) | VEHICLE (MIN) | I |
| I | 09.00 | . 15 |  |  |  |  |  |  |  |  | I |
| I | ARM A | 8.93 | 18.65 | 0.479 |  | 1.4 | 0.9 | 14.4 |  | 0.10 | I |
| I | ARM B | 7.10 | 18.26 | 0.389 |  | 0.9 | 0.6 | 9.9 |  | 0.09 | I |
| I | ARM C | 5.59 | 15.48 | 0.361 |  | 0.8 | 0.6 | 8.8 |  | 0.10 | I |
| I | ARM D | 2.42 | 14.16 | 0.171 |  | 0.3 | 0.2 | 3.2 |  | 0.09 | I |
| I |  |  |  |  |  |  |  |  |  |  | I |

## QUEUE AT ARM A

| TIME SEGMENT | NO. OF |
| :---: | ---: |
| ENDING | VEHICLES |
|  | IN QUEUE |


| 08.00 | 0.9 | $*$ |
| :--- | :--- | :--- |
| 08.15 | 1.4 | $*$ |
| 08.30 | 2.6 | $* *$ |
| 08.45 | 2.7 | $* * *$ |
| 09.00 | 1.4 | $*$ |
| 09.15 | 0.9 | $*$ |

QUEUE AT ARM B

| TIME SEGMENT | NO. OF <br> ENDING |
| :---: | ---: |
|  | VEHICLES <br> IN QUEUE |
| 08.00 |  |
| 08.15 | 0.6 |$\%$

## QUEUE AT ARM C

$\left.\begin{array}{crc}\text { TIME SEGMENT } \\ \text { ENDING }\end{array} \begin{array}{r}\text { NO. OF } \\ \text { VEHICLES } \\ \text { IN QUEUE }\end{array}\right]$

QUEUE AT ARM D
TIME SEGMENT

ENDING | NO. OF |
| :---: |
| VEHICLES |
| IN QUEUE |

| I | ARM | I | TOTAL DEMAND |  | $\begin{aligned} & \mathrm{I} \\ & \mathrm{I} \end{aligned}$ | * QUEUEING * |  | I | INCLUSIVE QUEUEING * <br> * DELAY * |  |  | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I |  |  | DE | AY * | I |  |  |  |  |
| I |  | I |  |  |  |  |  |  |  |  |  | I |
| I |  | I | (VEH) | (VEH/H) |  | I | (MIN) | (MIN/VEH) | I | (MIN) |  | (MIN/VEH) | I |
| I | A | I | 979.0 | I 652.7 | I | 145.7 I | 0.15 | I | 145.8 | I | 0.15 | I |
| I | B | I | 778.8 | I 519.2 | I | 85.0 I | 0.11 | I | 85.1 | I | 0.11 | I |
| I | C | I | 612.9 | I 408.6 | I | 81.2 I | 0.13 | I | 81.2 | I | 0.13 | I |
| I | D | I | 266.0 | I 177.3 | I | 27.9 I | 0.10 | I | 27.9 | I | 0.10 | I |
| I | ALL | I | 2636.8 | I 1757.9 | I | 339.9 I | 0.13 | I | 339.9 | I | 0.13 | I |

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
* INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD. * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

END OF JOB
$\qquad$
$\qquad$ ARCAD Y 6 $\qquad$
ASSESSMENT OF ROUNDABOUT CAPACITY AND DELAY

Analysis Program: Release 2.0 (MAR 2005)
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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

Run with file:-
"u:\CTB AOE 000 - Foxlydiate Lane, Webheath, Redditch\Ca1cs\Traffic Surveys\mode11ing\}
Final Modelling for Report \Junction $4 \backslash J u n c t i o n ~ 4-P r i m a r y ~ S i t e ~ A c c e s s ~ P M ~ P e a k . v a i " ~$ (drive-on-the-1eft ) at 14:01:53 on Thursday, 14 October 2010

FILE PROPERTIES
***************

RUN TITLE: Junction 4: Primary Site Access - PM Peak
LOCATION: Redditch
DATE: 17/09/2010
CLIENT: Heyford Developments
ENUMERATOR: ME
JOB NUMBER: CTBAOE
STATUS: Preliminary DESCRIPTION:

INPUT DATA
**********
ARM A - A448 from overbridge
ARM B - From offslip
ARM C - Link Road
ARM D - Site Access

GEOMETRIC DATA


V = approach half-width
$E=$ entry width

$$
\begin{aligned}
& \mathrm{L}=\text { effective flare length } \\
& \mathrm{R}=\text { entry radius }
\end{aligned}
$$

D = inscribed circle diameter
PHI = entry angle

| I ARM | I | FLOW | SCALE (\%) |
| :---: | :---: | :---: | :---: |
|  | I |  |  |
| I A | I | 100 | I |
| I | B | I | 100 |
| I C C | I | 100 | I |
| I | D | I | 100 |

DEMAND SET TITLE: PM Peak


DEMAND SET TITLE: PM Peak

| I |  | I | TURNING PROPORTIONS |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  | I | TURNING COUNTS (VEH/HR) |  |  |  |  |  |  |
| I |  | I |  |  |  | ERCENTAGE | OF H.V.S |  | I |
| I | TIME | I | FROM |  | I | ARM A I | ARM B I | ARM C I | ARM D I |
| I | 16.45-18.15 | I |  |  | I | I | I | I | I |
| I |  | I | ARM | A | I | 0.000 I | 0.742 I | 0.188 I | 0.070 I |
| I |  | I |  |  |  | 0.0 I | 530.0 I | 134.0 I | 50.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0)I | ( 10.0) I | ( 10.0) I |
| I |  | I |  |  | I | I | 0.000 | I |  |
| I |  | I | ARM | B | I | 0.488 I | 0.000 I | 0.365 I | 0.147 I |
| I |  | I |  |  | I | 372.0 I | 0.0 I | 278.0 I | 112.0 I |
| I |  | I |  |  | I | ( 10.0)I | ( 10.0)I | ( 10.0)I | ( 10.0)I |
| I |  | I |  |  | I | I | ( | I |  |
| I |  | I | ARM | C | I | 0.554 I | 0.362 I | 0.000 I | 0.085 I |
| I |  | I |  |  | I | 150.0 I | 98.0 I | 0.0 I | 23.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0)I | ( 10.0) I | ( 10.0) I |
| I |  | I |  |  | I | I | I | I |  |
| I |  | I | ARM | D | I | 0.542 I | 0.336 I | 0.121 I | 0.000 I |
| I |  | I |  |  | I | 58.0 I | 36.0 I | 13.0 I | 0.0 I |
| I |  | I |  |  | I | ( 10.0) I | ( 10.0)I | ( 10.0)I | ( 10.0) I |
| I |  | I |  |  | I | I | I | I |  |

QUEUE AND DELAY INFORMATION FOR EACH 15 MIN TIME SEGMENT




## QUEUE AT ARM

| TIME SEGMENT | NO. OF |
| :---: | ---: |
| ENDING | VEHICLES |
|  | IN QUEUE |


| 17.00 | 0.8 | $*$ |
| :--- | :--- | :--- |
| 17.15 | 1.2 | $*$ |
| 17.30 | 2.1 | $* *$ |
| 17.45 | 2.2 | $* *$ |
| 18.00 | 0.3 | $*$ |
| 18.15 |  |  |

QUEUE AT ARM B

| TIME SEGMENT ENDING | NO. OF VEHICLES IN QUEUE |  |
| :---: | :---: | :---: |
| 17.00 | 1.2 | * |
| 17.15 | 1.8 | ** |
| 17.30 | 4.1 | **** |
| 17.45 | 4.3 | **** |
| 18.00 | 2.0 | ** |
| 18.15 | 1.2 | * |

QUEUE AT ARM C

| TIME SEGMENT | NO. OF <br> ENDING |
| :---: | :---: |
|  | VEHICLES <br> IN QUEUE |
| 17.00 |  |
| 17.15 | 0.3 |
| 17.30 | 0.4 |
| 17.45 | 0.6 |$*$

QUEUE AT ARM D

| TIME SEGMENT <br> ENDING | NO. OF <br> VEHICLES <br> IN QUEUE |
| :---: | :---: |
|  |  |
| 17.00 | 0.1 |
| 17.15 | 0.1 |
| 17.30 | 0.2 |
| 17.45 | 0.2 |
| 18.00 | 0.1 |
| 18.15 | 0.1 |


| I | ARM | $\begin{aligned} & I \\ & I \\ & I \\ & I \\ & I \end{aligned}$ | TOTAL DEMAND |  | I | $\begin{aligned} & * \text { QUEUEING } * \\ & * \text { DELAY } * \end{aligned}$ |  | I | INCLUSIVE QUEUEING <br> * DELAY * |  |  | I |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| I |  |  |  |  | I |  |  |  |  |  |  |
| I |  |  | (VEH) | (VEH/H) |  | I | (MIN) | (MIN/VEH) | I | (MIN) |  | (MIN/VEH) | I |
| I | A | I | 979.0 I | I 652.7 | I | 124.9 | 0.13 | I | 124.9 | I | 0.13 | I |
| I | B | I | 1044.9 I | I 696.6 | I | 211.0 I | 0.20 | I | 211.1 | I | 0.20 | I |
| I | C | I | 371.6 I | I 247.7 | I | 37.3 I | 0.10 | I | 37.3 | I | 0.10 | I |
| I | D | I | 146.7 I | I 97.8 | I | 11.2 I | 0.08 | I | 11.2 | I | 0.08 | I |
| I | ALL | I | 2542.2 I | I 1694.8 | I | 384.4 I | 0.15 | I | 384.5 | I | 0.15 | I |

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END OF JOB


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